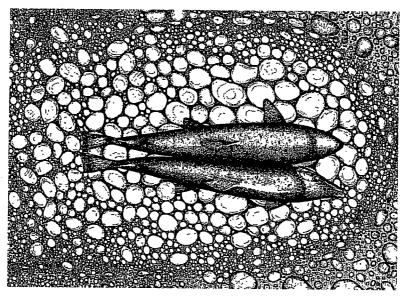
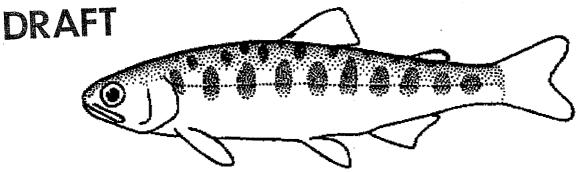
RELATIONSHIPS BETWEEN FLOW FLUCTUATIONS AND REDD DEWATERING AND JUVENILE STRANDING FOR CHINOOK SALMON AND STEELHEAD IN THE SACRAMENTO RIVER BETWEEN KESWICK DAM AND BATTLE CREEK





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DRAFT

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CVPIA INSTREAM FLOW INVESTIGATIONS SACRAMENTO RIVER BETWEEN KESWICK DAM TO BATTLE CREEK CHINOOK SALMON AND STEELHEAD REDD DEWATERING AND JUVENILE STRANDING

PREFACE

The following is the final report for the U. S. Fish and Wildlife Service's investigations on the effects of flow fluctuations on anadromous salmonid redd dewatering and juvenile stranding in the Sacramento River between Keswick Dam and Battle Creek. These investigations are part of the Central Valley Project Improvement Act (CVPIA) Instream Flow Investigations, a 7-year effort which began in February, 1995. Title 34, Section 3406(b)(1)(B) of the CVPIA, P.L. 102-575, requires the Secretary of the Interior to determine instream flow needs for anadromous fish for all Central Valley Project controlled streams and rivers, based on recommendations of the U. S. Fish and Wildlife Service after consultation with the California Department of Fish and Game (CDFG). The purpose of these investigations are to provide scientific information to the U. S. Fish and Wildlife Service Central Valley Project Improvement Act Program to assist in developing such recommendations for Central Valley rivers.

Written comments or questions about this report or these investigations should be submitted to:

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ACKNOWLEDGMENTS

The field work for this study was conducted by Will Amy, Ed Ballard, Brian Cordone, Mark Gard, John Kelly, Jason Kent, Jeff Thomas, Erin Sauls, Jerry Big Eagle and Rick Williams. Data analysis and report preparation were performed by Mark Gard. Funding was provided by the Central Valley Project Improvement Act.

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INTRODUCTION

In response to substantial declines in anadromous fish populations, the Central Valley Project Improvement Act requires the doubling of the natural production of anadromous fish stocks, including the four races of chinook salmon (fall, late-fall, winter, and spring runs), steelhead, and white and green sturgeon. For the Sacramento River, the Central Valley Project Improvement Act Anadromous Restoration Plan calls for October through April flows ranging from 3,250 to 5,500 cfs, with the recommended flow varying with the October 1 carryover storage in Shasta Reservoir (U. S. Fish and Wildlife Service 1995). In December 1994, the U. S. Fish and Wildlife Service prepared a study proposal to identify the instream flow requirements for anadromous fish in certain streams within the Central Valley of California, including the Sacramento River. The purpose of this report is to model the effects of flow fluctuations on chinook salmon and steelhead redd dewatering and juvenile entrapment stranding in the Sacramento River between Keswick Reservoir and Battle Creek.

A 2-dimensional hydraulic and habitat model (RIVER2D) was used for the redd dewatering portion of this modeling, instead of the Physical Habitat Simulation (PHABSIM¹) component of the Instream Flow Incremental Methodology (IFIM). The 2-D model uses as inputs the bed topography and substrate of a site, total discharge at the upstream transect, and the water surface elevation at the downstream transect of the site, to predict the amount of habitat present in the site. The 2-D model avoids problems of transect placement, since the entire site can be modeled. The 2-D model also has the potential to model depths and velocities over a range of flows more accurately than PHABSIM because it takes into account upstream and downstream bed topography and bed roughness, and explicitly uses mechanistic processes (conservation of mass and momentum), rather than Manning's n and a velocity adjustment factor.

Other advantages of 2-D modeling are that it can explicitly handle complex habitats, including transverse flows, across-channel variation in water surface elevations, and flow contractions/expansions. The model scale is small enough to correspond to the scale of microhabitat use data with depths and velocities produced on a continuous basis, rather than in discrete cells. The 2-D model does a better job of representing patchy microhabitat features, such as gravel patches. The data can be collected with a stratified sampling scheme, with higher intensity sampling in areas with more complex or more quickly varying microhabitat features, and lower intensity sampling in areas with uniformly varying bed topography and uniform substrate. Bed topography and substrate mapping data can be collected at a very low flow, with the only data needed at high flow being water surface elevations at the upstream and downstream transects of the site and flow and edge velocities for validation purposes. In addition, alternative habitat suitability criteria, such as measures of habitat diversity, can be used.

¹ PHABSIM is the collection of one dimensional hydraulic and habitat models which are used to predict the relationship between physical habitat availability and streamflow over a range of river discharges.

The results of this study are intended to support or revise the flow recommendations above.

METHODS

Study Site Selection

We have divided the Sacramento River study area into six stream segments (Figure 1), based on hydrology and other factors: Grimes to Colusa (Segment 1); Deer Creek to Red Bluff Diversion Dam (Segment 2); above Lake Red Bluff to Battle Creek (Segment 3); Battle Creek to Cow Creek (Segment 4); Cow Creek to ACID (Segment 5); and ACID to Keswick Dam (Segment 6). Segment one addresses green and white sturgeon, while the other segments address chinook salmon and steelhead.

CDFG conducted mesohabitat mapping of the Sacramento River between Keswick Dam and Battle Creek. CDFG used thirteen mesohabitat types: bar complex glides, bar complex pools, bar complex riffles, bar complex runs, flatwater glides, flatwater pools, flatwater riffles, flatwater runs, side channel glides, side channel pools, side channel riffles, side channel runs, and off-channel areas (Snider et al 1992). The mesohabitat units (MHUs) were designated with numbers, starting with MHU # 5 at Battle Creek to MHU # 143 just below Keswick Dam.

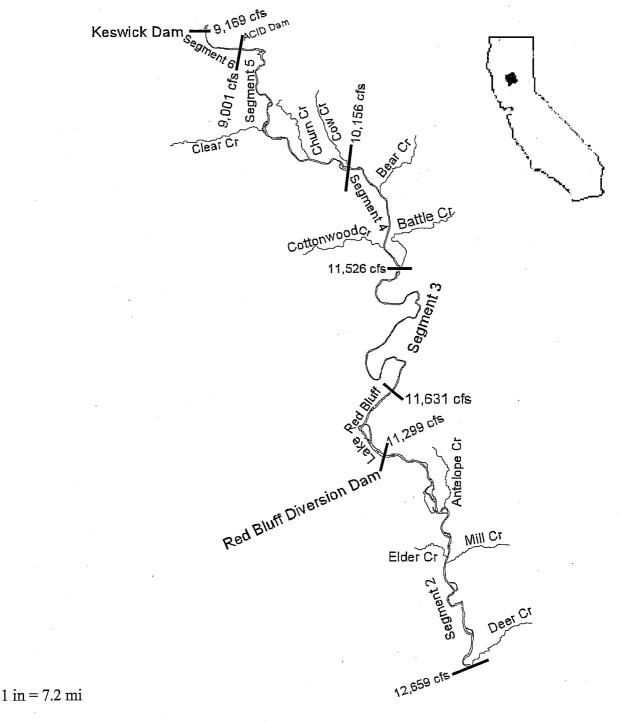
The redd dewatering analysis was conducted using data from our eight spawning sites (Lower Lake Redding, Upper Lake Redding, Salt Creek, Bridge Riffle, Posse Grounds, Above Hawes Hole, Powerline Riffle and Price Riffle). Information on these sites is given in U.S. Fish and Wildlife Service 1999.

We surveyed both banks of the Sacramento River from Keswick Dam to Battle Creek to identify locations where juvenile chinook salmon could become trapped in inundated areas isolated from the main river channel when Sacramento River flows drop. Our surveys were conducted at relatively low flows (less than 8,000 cfs). The criteria that we used to identify stranding areas were: 1) the area would not drain to the main river channel; 2) the area would strand at river flows ranging from 3,250 to 15,000 cfs; and 3) the area was not the mouth of a tributary. We found 107 locations which would potentially become isolated from the main channel at flows ranging from 3,250 to 15,000 cfs. Twenty-seven of these sites were identified in October 1998. The remaining sites were identified in December 1999 and January and April 2000. The location of these sites are identified in Appendix A.

Transect Placement (study site setup)

Details on transect placement for the spawning sites are given in U.S. Fish and Wildlife Service 1999. Details on site setup for our juvenile habitat modeling sites (used as discussed below for some of the stranding sites) are given in U.S. Fish and Wildlife Service 2005.

Figure 1
Sacramento River Stream Segments 2-6²



² Flows are the average flows for the period October 1974 to September 1993 at the top of each segment.

Three main approaches were used to determine the stranding flows³ for the 107 stranding sites:

1) for those stranding sites located in one of our juvenile habitat modeling sites, the

2-dimensional hydraulic model of the juvenile habitat site was used to determine the stranding flow for the stranding site; 2) for those stranding sites where the flow during our identification of the stranding site was at or slightly above or below the stranding flow for that site, we determined the stranding flow based on the flow on that date; and 3) for the remaining sites, we developed a stage-discharge relationship for the main river channel at the stranding site to determine the stranding flow. There were 10 stranding sites in our juvenile habitat modeling sites, 44 sites where the flow during our identification of the stranding site was at or slightly above or below the stranding flow for that site, and 53 sites for which we developed stage-discharge relationships. The first two categories of sites did not require any site setup or data collection, while the third category of site required the installation of a vertical benchmark (a lag bolt in a tree).

Hydraulic and Structural Data Collection

Areas were determined for all of the stranding sites. For smaller sites, we determined the area by measuring the length and two to six widths of the stranding site, using an electronic distance meter; the area is calculated by multiplying the length times the average width. The areas of larger sites were measured on aerial photos or output of the RIVER2D modeling of our juvenile habitat modeling sites using a planimeter, or for Site 45B, using digitized aerial photos in GIS.

Vertical benchmarks were established at each of the 53 stranding sites for which we developed flow-habitat relationships to serve as the reference elevations to which all elevations (streambed and water surface) were tied. Vertical benchmarks consisted of lag bolts driven into trees.

Data required for developing a stage discharge relationship are: 1) water surface elevations (WSELs, stages), measured to the nearest 0.01 foot at three flows using standard surveying techniques (differential leveling); and 2) the stage of zero flow (SZF). Water surface elevations were measured at all but one of the 53 stranding sites for which we developed flow-habitat relationships at the following three flow ranges: 4,700-8,000 cfs, 6,000-12,000 cfs and 11,800-15,100 cfs (Appendix B)⁴. For one site, WSELs were measured at four different flows, with two

³ We defined the stranding flow as the flow where the connection between the stranding area and main river channel has a maximum depth of 0.1 feet. We selected 0.1 feet because the minimum depth at which we found juvenile salmon during our juvenile HSI data collection was 0.2 feet. When flows drop to or below the stranding flow, juvenile salmon will be isolated from the main river channel.

⁴ For the remaining site, we were only able to measure WSELs at two flows (10,181 and 14,986 cfs, since this site was located on a side channel which stopped flowing when the total river flow dropped below 9,300 cfs.

flows in the first flow range. We also measured the bed elevation of the stranding point (the lowest point at the connection between the stranding area and the main river channel) using differential leveling; the stage at the stranding flow was calculated by adding 0.1 feet to the bed elevation of the stranding point. After the stage discharge relationship was developed, it was used to determine what the flow is at the stranding flow stage. For most of the sites, the SZF was determined by making a traverse with a 600 kHz Broad-Band Acoustic Doppler Current Profiler (ADCP) across the main channel at the stranding point. For a few sites on side channels where the entire channel could be waded, the SZF was determined by measuring depths across the side channel with a wading rod. In both cases, the SZF was calculated as the difference between the WSEL on that date and the largest depth.

Flows for most sites were determined from gage data. There were 12 of the 53 stranding sites that were located on split channels. For 4 of these sites, we used flow/flow regressions between the split channel flow and the total Sacramento River discharge developed for the juvenile rearing habitat modeling sites. For the remaining 8 sites located on split channels, flows were measured when the WSELs were collected, to enable the development of flow/flow regressions between the split channel flow and the total Sacramento River discharge. For sites where the entire channel was wadable, flows were measured by making depth and velocity measurements by wading with a wading rod equipped with a Marsh-McBirney^R model 2000 or a Price AA velocity meter. For deeper sites, depth and velocity measurements in portions of the channel with depths greater than 3 feet were made with the ADCP, while depth and velocity measurements in shallower areas were made by wading with a wading rod equipped with a Marsh-McBirney^R model 2000 or a Price AA velocity meter. The ADCP settings used are shown in Table 1. Starting at the water's edge, water depths and velocities were made at measured intervals using the wading rod and Marsh-McBirney^R model 2000 or Price AA velocity meter until the water became sufficiently deep to operate the ADCP (approximately 3 feet). The distance intervals of each depth and velocity measurement from the water's edge were measured using a hand held laser range finder. At the location of the last depth and velocity measurement made while wading, a buoy was placed to serve as a starting point for the ADCP. The boat was then positioned so that the ADCP started operation at the buoy, and water depth and velocity data were collected across the channel to the location near the opposite bank where water depths of approximately 3 feet were reached. A buoy was placed at the location where ADCP operation ceased and the procedure used for measuring depths and velocities in shallow water was repeated until the far bank water's edge was reached. Additional details on the ADCP operation are given in Gard and Ballard (2003).

Data collection started in October 1998 and was completed in August 2001.

Table 1
CFG Files⁵ Used for ADCP Data

CFG File	Mode	Depth Cell Size (ft)	Depth Cell Number	Max Bottom Track Depth (ft)	Pings	WT ⁶	First Depth Cell (ft)	Blanking Dist. (ft)
MD8A	8	0.66	15	26	4	5	1.61	0.33
MD4C	4	0.33	30	26	4	5	1.51	0.33
MD4E	4	0.66	30	26	4	5	1.84	0.33
MD4H	4	0.66	100	52	4	5	1.84	0.33
D45D	8	0.66	30	26	4	5	1.94	0.66

Our eight spawning sites were originally modeled with PHABSIM (U.S. Fish and Wildlife Service 1999). Seven of these sites were subsequently modeled with RIVER2D for our juvenile habitat modeling (U.S. Fish and Wildlife Service 2005). We measured the horizontal location of the head and tail pins of the transects at the remaining spawning site, Bridge Riffle, with a total station, so that we could model this site with RIVER2D, using all of the points on these transects to determine the bed topography and substrate of this site. We collected 383 data points on the Bridge Riffle site transects, corresponding to a density of 3.9 points/100 m².

Hydraulic Model Construction and Calibration

ASCII files of each ADCP traverse for flow or SZF measurements were produced using the Playback feature of the Transect program⁷. Each ASCII file was then imported into the Riverine Habitat Simulation (RHABSIM)⁸ Version 2.0 to produce the bed elevations, the component of the average water column velocities perpendicular to the transect, and stations (relative to the

⁵ The first four characters of the ADCP traverses designates which CDG file (containing the ADCP settings) was used for the traverses.

⁶ WT is the water track transmit length.

⁷ The Transect program is the software used to receive, record and process data from the ADCP.

⁸ RHABSIM is a commercially-produced software (Payne and Associates 1998) that incorporates the modeling procedures used in PHABSIM.

start of the ADCP traverse). RHABSIM was then used to output a second ASCII file containing this data. For the SZF measurements, the second ASCII file was input into a QuattroPro spreadsheet where the maximum depth was subtracted from the measured WSEL to compute the SZF (Appendix B).

For the ADCP traverses made for flow measurements, the second ASCII file was input into a QuattroPro spreadsheet and combined with the velocity, depth, and station data collected in shallow water. We defined a statistic (R) to provide a quality control check of the velocity measured by the ADCP at a given station n, where $R = Vel_n/(Vel_{n-1} + Vel_{n+1})/2$ at station n^9 . R was calculated for each velocity where Vel_n , Vel_{n-1} and Vel_{n+1} were all greater than 1 ft/s for each ADCP data set. Based on data collected using a Price AA velocity meter on the Lower American River, the acceptable range of R was set at 0.5-1.6. All verticals with R values less than 0.5 or greater than 1.6 were deleted from each ADCP data set. Discharges were calculated for each ADCP traverse, including the data collected in shallow water.

Flow/flow regressions were performed for sites which did not include the entire Sacramento River flow (Stranding Sites 14, 66, 87-90 and 97), using the flows measured in the site, and the corresponding total flows determined from gage readings¹¹ (Table 2). For Stranding Sites 87-90 and 97, the regressions were developed from three sets of flows, with the entire river discharge around 6,000 cfs, 9,000-10,000 cfs and 15,000 cfs. For Stranding Site 66, the regressions were only developed from two sets of flows, at 10,181 and 14,986 cfs, since the side channel on which this site was located stopped flowing when the total river flow dropped below 9,300 cfs. For Stranding Site 14, the regressions were also only developed from two sets of flows, at 6,152 and 8,826 cfs, made by wading the channel, since the channel could not be waded at flows above 9,000 cfs, and the channel was too shallow for accurate ADCP measurements. The site flows used in the regression were either the average of the ADCP traverses at the site or the flows measured with a wading rod and Price AA or Marsh-McBirney meter on the site. Calibration flows for Stranding Sites 14, 66, 87-90 and 97 (Table 4) were computed from the total discharge in Table 2 and the appropriate regression equation in Table 3.

 $^{^{9}}$ n - 1 refers to the station immediately before station n and n + 1 refers to the station immediately after station n.

 $^{^{10}}$ We also deleted velocities where Vel_n was less than 1.00 ft/s and Vel_{n-1} and Vel_{n+1} were greater than 2.00 ft/s, and where Vel_n had one sign (negative or positive) and Vel_{n-1} and Vel_{n+1} had the opposite sign (when the absolute value of all three velocities were greater than 1.00 ft/s); these criteria were also based on the Lower American River dataset (Gard and Ballard 2003).

¹¹ As shown in Table 2, the flow calculated at Bend Bridge from upstream and tributary gage readings often differed from the gage reading at Bend Bridge by less than 5% and never differed by more than 10%. Flows could be calculated using either USBR or USGS flows measured at Keswick Dam; the flows selected for use were those which had the smaller Bend error.

Table 2 Sacramento River Flows at Stranding Study Sites¹² (cfs)

			Strano	ling Study	Site Nu	mber			_	
Date	1-8	9-22 & 27-32	23-26, 33- 34, 39-52, 55-56	35-38, 53- 54, 57, 59-70	58, 71- 79C, 83- 90	80-81, 91, 96- 97	92-94	82, 95	Bend error	Keswick Flow Used
10/13/98	6580								7.64%	USGS
10/14/98		6152	6361						9.19%	USGS
5/25/99	10045	9813	9753				,		2.53%	USBR
8/5/99	12032	11822	11762						0.02%	USBR
12/9/99		7683	7683	7898					0.74%	USBR
12/10/99			7554						1.53%	USBR
1/12/00		4710	4710	4936					5.51%	USGS
3/17/00		11700	11700	12009					9.66%	USGS
4/25/00				8994					2.48%	USGS
4/26/00				8700	8926	9428			2.74%	USGS
4/27/00			8608		9035		9606		3.17%	USGS
7/10/00		14990		•					0.43%	USGS
7/11/00		14987	14927	14986					0.32%	USGS
7/12/00					14988	15409	•		0.43%	USGS
7/13/00							15071		0.79%	USGS
3/13/01			6150						5.55%	USGS
3/14/01				6086	6244				4.52%	USGS
3/15/01					5977	6454	6511		4.36%	USGS
7/9/01					14580				2.32%	USGS
8/20/01				,	10181				2.23%	USGS
8/22/01						9239			2.33%	USGS

See U.S. Fish and Wildlife Service 1999 and U.S. and Fish and Wildlife Service 2005, respectively, for details on the hydraulic model construction and calibration on the spawning site PHABSIM transects and our juvenile habitat modeling RIVER2D sites.

¹² These flows are the same as the stranding study site flows for those stranding sites that include all of the Sacramento River flow.

Table 3 Flow/Flow Regression Equations

Stranding Study Site	Regression Equation ¹³	R²-value
Site 14	Site $14 Q = -377 + 0.1365 \times Q$	114
Site 66	Site $66 Q = -542 + 0.0583 \times Q$	114
Sites 87-90	Sites $87-90 Q = -1976 + 0.3566 \times Q$	0.9655
Site 97	Site $97 Q = -1424 + 0.5696 \times Q$	0.99999

Table 4
Calibration Flows for Stranding Study Sites 14, 66, 87-90 and 97 (cfs)

Date	Site 14	Site 66	Sites 87-90	Site 97
10/14/98	463			
5/25/99	963			
8/5/99	1237			
7/11/00		331		
7/12/00			3368	7148
3/14/01			201	
3/15/01			155	2252
8/20/01		51	1654	
8/22/01		•		3839

All stage-discharge data were compiled and checked before entry into PHABSIM data decks for the 53 stranding sites for which we developed flow-habitat relationships. A total of two to four sets of WSELs at widely spaced flows were used. Calibration flows in the data decks were the flows calculated from gage readings or the flows calculated from gage readings and the regression equations in Table 3. A separate deck was constructed for each set of study sites with the same calibration flows.

¹³ Q is the total river flow, Site 14 Q is the flow in Stranding Site 14, etc.

Since only two flows were used in these regressions, the R²-values, by definition, were one.

The first step in the calibration procedure was to determine the best approach for WSEL simulation. Initially, the *IFG4* hydraulic model (Milhous *et al.*, 1989) was run on each deck to compare predicted and measured WSELs. This model produces a stage-discharge relationship using a log-log linear rating curve calculated from at least three sets of measurements taken at different flows. *IFG4* is considered to have worked well if the following criteria are met: 1) the beta value (a measure of the change in channel roughness with changes in streamflow) is between 2.0 and 4.5; 2) the mean error in calculated versus given discharges is less than 10%; 3) there is no more than a 25% difference for any calculated versus given discharge; and 4) there is no more than a 0.1 foot difference between measured and simulated WSELs¹⁵. For a majority of the sites, *IFG4* met the above criteria for *IFG4* (Appendix B).

For those transects/flow ranges modeled with *IFG4*, the mean error and calculated-given discharge criteria were met in all cases, and the measured-simulated WSEL difference criterion for *IFG4* was met in all cases except for stranding sites 15, 16, 30, 39, 46 and 77. We still used *IFG4* for these sites because: 1) the difference between measured and simulated WSELs for all sites was less than 0.19 foot¹⁶; 2) in all cases the stranding flow was not greater than the highest calibration flow; and 3) the calibration plots indicated that there was a linear log-log relationship over the range of calibration flows.

For stranding sites 57, 87, 88A and 90, the initial *IFG4* calibration indicated that there was a significantly non-linear log-log relationship between stage and flow over the range of calibration flows. In these cases, we used a modification of *IFG4* where we only used two calibration flows. The calibration flows selected were those which bracketed the stranding flow. While this technique is not accepted for developing stage-discharge relationships, we concluded that it was sufficiently accurate for interpolating a stranding flow in between two calibration flows. Since only two flows are used in this method, the mean error and calculated versus given discharge criteria of *IFG4* do not apply and the difference between measured and predicted WSELs will always be zero.

As shown in Appendix B, the beta coefficient values were less than 2.0 for stranding sites 15, 57, 72, 74, 81 and 88A. We concluded that this phenomenon were caused by channel characteristics which form hydraulic controls at some flows but not at others (compound controls), thus affecting upstream water elevations. Specifically, at lower flows the channel at these sites controlled the water surface elevations, while at higher flows the water surface elevations were controlled by downstream hydraulic controls. Accordingly, the performance of *IFG4* for these sites was considered adequate despite the beta coefficient criterion not being met.

¹⁵ The first three criteria are from U.S. Fish and Wildlife Service 1994, while the fourth criterion is our own.

¹⁶ For much of the Sacramento River, the WSEL going across the river will differ by more than 0.2 feet.

As shown in Appendix B, the beta coefficient values were greater than 4.5 for stranding sites 11, 13, 37/38, 69/70, 87, 88 and 97. We concluded that this phenomenon was caused by the presence of a downstream hydraulic control, such that the actual SZFs of these sites were greater than those in Appendix B. We determined that the correct SZF would have had a minimal effect on the estimated stranding flows for these sites - for example, a SZF which produced a beta coefficient of 4.5 for stranding site 38 would have only increased the stranding flow from 13,771 cfs to 13,775 cfs. As a result, we concluded that the SZFs in Appendix B were sufficiently accurate for the purposes of estimating stranding flows.

There were three other sites (stranding sites 36, 66 and 75) for which we developed stage-discharge relationships using methods other than IFG4. Sites 36 and 75 were located in bar complexes where there was a significant variation in WSEL across the entire river; IFG4 can not be used in this case since a basic assumption of IFG4 is that the WSEL does not vary across the channel. For site 36, we developed a stage-discharge relationship using a regression of log(WSEL) versus log(flow), with the three measured WSELs at 4,936, 6,086 and 12,009 cfs. There was a significantly non-log-linear relationship between stage and flow for site 75 over the range of measured WSELs; since the stranding flow for this site ended up being less than 3,250 cfs, we used the two lower measured WSELs to estimate the stranding flow. We used a regression equation of the form $log(WSEL - A) = B + C \times log(flow)$ for site 75, where we determined A in the field by subtracting the maximum depth in the main channel from the WSEL measured at the stranding location¹⁷; B and C were derived from the regression. For site 66, we developed stage-discharge relationships using a regression of log(WSEL-SZF) versus log(flow), but only using two flows (14,986 cfs and 10,181 cfs). As discussed above, we only measured WSELs at two flows for site 66. The regression equations for these sites are given in Table 5.

Table 5
Stage-Discharge Regression Equations

Stranding Study Site	Regression Equation ¹⁸	R²-value
Site 36	$log (WSEL) = 1.913 + 0.01979 \times log (Q)$	0.9997
Site 66	$\log (WSEL - 96.0) = -0.506 + 0.376 \times \log (Site 66 Q)$	119
Site 75	$log (WSEL - 80.8) = 0.7670 + 0.0755 \times log (Q)$	119

¹⁷ A is similar to a SZF, but is not strictly speaking a SZF since the WSEL at the location with the maximum depth was significantly lower than the WSEL at the stranding location.

¹⁸ Q is the total river flow, Site 66 Q is the flow in Stranding Site 66, etc.

¹⁹ Since only two flows were used in these regressions, the R²-values, by definition, were one.

The stranding flows for the 107 stranding sites are given in Appendix A. Sites 1-8, located upstream of ACID, had different stranding flows depending on whether the ACID dam was in or out. The stage-discharge relationships for 13 of the stranding sites resulted in a stranding flow of less than 3,250 cfs; these sites were thus dropped from consideration, since we are identifying areas that strand at flows between 3,250 and 15,000 cfs. Another two sites were dropped from consideration because they stranded at flows significantly greater than 15,000 cfs.

For the Bridge Riffle site, the PHABSIM transect data was used in QuattroPro to create the input files (bed and substrate) for the 2-D modeling program. An artificial extension one channel-width-long was added upstream of the top of the site to enable the flow to be distributed by the model when it reached the study area, thus minimizing boundary conditions influencing the flow distribution at the upstream transect and within the study site. The bed files contain the horizontal location (northing and easting), bed elevation and initial bed roughness value for each point, while the substrate files contain the horizontal location, bed elevation and substrate code for each point. The initial bed roughness value for each point was determined from the substrate codes for that point and the corresponding bed roughness values in Table 6. The bed roughness values in Table 6 were computed as five times the average particle size. The bed and substrate files were exported from QuattroPro as ASCII files.

A utility program, R2D_BED (Steffler 2001b), was used to define the study area boundary and to refine the raw topographical data TIN (triangulated irregular network) by defining breaklines²⁰ following longitudinal features such as thalwegs, tops of bars and bottoms of banks. Breaklines were also added along lines of constant elevation. The bed topography of Bridge Riffle site is shown in Figure 2.

An additional utility program, R2D_MESH (Steffler 2001a), was used to define the inflow and outflow boundaries and create the finite element computational mesh for the River2D model. R2D_MESH uses the final bed files as an input. The first stage in creating the computational mesh was to define mesh breaklines²¹ which coincided with the final bed file breaklines. Additional mesh breaklines were then added between the initial mesh breaklines, and additional nodes were added as needed to improve the fit between the mesh and the final bed file and to improve the quality of the mesh, as measured by the Quality Index (QI) value. The QI is a

²⁰ Breaklines are a feature of the R2D_Bed program which force the TIN of the bed nodes to linearly interpolate bed elevation and bed roughness values between the nodes on each breakline and force the TIN to fall on the breaklines (Steffler 2001b).

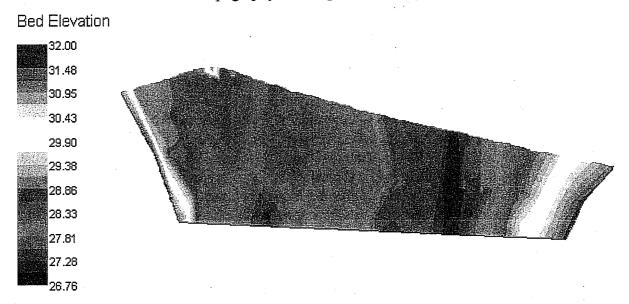
²¹ Mesh breaklines are a feature of the R2D_MESH program which force edges of the computation mesh elements to fall on the mesh breaklines and force the TIN of the computational mesh to linearly interpolate the bed elevation and bed roughness values of mesh nodes between the nodes at the end of each breakline segment (Steffler 2001a). A better fit between the bed and mesh TINs is achieved by having the mesh and bed breaklines coincide.

Table 6
Initial Bed Roughness Values

Substrate Code	Туре	Particle Size (inches)	Bed Roughness (m)
0.1	Sand/Silt	< 0.1	0.05
1	Small Gravel	0.1 - 1	0.1
1.2	Medium Gravel	1 - 2	0.2
1.3	Medium/Large Gravel	1 - 3	0.25
2.3	Large Gravel	2 - 3	0.3
2.4	Gravel/Cobble	2 - 4	0.4
3.4	Small Cobble	3 - 4	0.45
3.5	Small Cobble	3 - 5	0.5
4.6	Medium Cobble	4 - 6	0.65
6.8	Large Cobble	6 - 8	0.9
8	Large Cobble	8 - 10	1.25
9	Boulder/Bedrock	> 12	0.05
10	Large Cobble	10-12	1.4
			·

measure of how much the least equilateral mesh element deviates from an equilateral triangle. An ideal mesh (all equilateral triangles) would have a QI of 1.0. A QI value of at least 0.2 is considered acceptable (Steffler 2001a). The mesh for the Bridge Riffle site, with 4819 nodes, had a QI value of 0.30. The percentage of the original bed nodes for which the mesh differed by less than 0.1 foot (0.03 m) from the elevation of the original bed nodes was 87% for the Bridge Riffle site. In most cases, the areas of the mesh where there was greater than a 0.1 foot (0.03 m) difference between the mesh and final bed file were in steep areas; in these areas, the mesh would be within 0.1 foot (0.03 m) vertically of the bed file within 1 foot (0.3 m) horizontally of the bed file location. Given that we had a 1-foot (0.3 m) horizontal level of accuracy, such areas would have an adequate fit of the mesh to the bed file. The final step with the R2D_MESH software was to generate the computational (cdg) file.

Figure 2
Bed Topography of Bridge Riffle Study Site



Units of Bed Elevation are meters.

The cdg file was opened in the RIVER2D software, where the computational bed topography mesh was used together with the WSEL at the bottom of the site, the flow entering the site, and the bed roughnesses of the computational mesh elements to compute the depths, velocities and WSELs throughout the site. The basis for the current form of RIVER2D is given in Ghanem *et al* (1995). The computational mesh was run in RIVER2D to steady state at a mid-range flow (15,149 cfs) for which WSELs were measured, and the WSELs predicted by RIVER2D at the upstream end of the site were compared to the WSELs measured at both upstream transects. The bed roughnesses of the computational mesh elements were then modified by multiplying them by a constant bed roughness multiplier (BR Mult) until the WSELs predicted by RIVER2D at the upstream transect locations matched the WSELs measured at both upstream transects. The values of all other River2D hydraulic parameters were left at their default values (upwinding coefficient = 0.5, minimum groundwater depth = 0.05 m, groundwater transmissivity = 0.1, groundwater storativity = 1, and eddy viscosity parameters epsilon 1 = 0.01, epsilon2 = 0.5 and epsilon3 = 0.1).

A stable solution will generally have a solution change (Sol Δ) of less than 0.00001 and a net flow (Net Q) of less than 1% (Steffler and Blackburn 2001). In addition, solutions for low gradient streams should usually have a maximum Froude Number (Max F) of less than one²². Finally, the WSEL predicted by the 2-D model should be within 0.10 foot (0.031 m) of the

²² This criteria is based on the assumption that flow in low gradient streams is usually subcritical, where the Froude number is less than one (Peter Steffler, personal communication).

WSEL measured at the upstream transects²³. The calibrated cdg file for Bridge Riffle site, with a BR Mult of 0.3, had a Sol Δ of 2 x 10⁻⁷, a net Q of 0.2%, and a Max F of 1.00. To get the Bridge Riffle site to calibrate, we needed to lower the downstream WSEL by 0.1 foot; the downstream WSELs for the simulation flows were also lowered by 0.1 foot. This allowed us to calibrate the model while still having the WSEL at the downstream transect within 0.1 foot of the measured WSEL. The Bridge Riffle site calibrated cdg file had WSELs that differed by more than 0.1 foot (0.031 m) from the measured WSELs for transects 2 and 3 main channel²⁴ (Table 7). For transect 2, the predicted WSELs near the water's edge, where the WSELs were measured, were within 0.1 foot (0.031 m) of the measured WSELs. For much of the Sacramento River, the WSEL going across the river will differ by more than 0.1 foot (0.031 m), with up to a 3-foot (0.91 m) measured difference in WSEL between the two banks in some areas, such as the Posse Grounds site. Accordingly, we conclude that the calibration for these transects was acceptable. For transect 3 main channel, the simulated WSEL on the left bank only differed by 0.18 foot (0.054 m) from the measured WSEL, and this was only at a small area right at the water's edge. Given the above discussion, we conclude that the WSEL calibration of Bridge Riffle site was acceptable.

Table 7
2-D WSEL Calibration Statistics

	Difference (measured versus predicted WS	ELs, ft) ²⁵
Transect	Average	Standard Deviation	Maximum
1	0.10		0.10
2	0.03	0.28	0.49
2 LB	0.07	0.01	0.08
3 SC	0.07	0.002	0.08
3 MC	0.30	0.15	0.49
3 MC LB	0.09	0.04	0.18

²³ We have selected this standard because it is a standard used for PHABSIM (U. S. Fish and Wildlife Service 2000).

²⁴ Bridge Riffle transect 3 had both a side channel (SC) and a main channel (MC).

²⁵ All WSEL measurements were made on the left bank (LB).

Velocity validation is the final step in the preparation of the hydraulic models for use in habitat simulation. Velocities predicted by RIVER2D were compared with measured velocities to determine the accuracy of the model's predictions of mean water column velocities. The measured velocities used were the velocities measured on the three transects. See Appendix C for velocity validation statistics. Although there was a strong correlation between predicted and measured velocities, there were significant differences between individual measured and predicted velocities. In general, the simulated and measured velocities profiles at the three transects (Appendix C²⁶) were relatively similar in shape. Differences in magnitude in most cases are likely due to: (1) operator error during data collection, i.e., the probe was not facing precisely into the direction of current; (2) range of natural velocity variation at each point over time resulting in some measured data points at the low or high end of the velocity range averaged in the model simulations; (3) aspects of the bed topography of the site that were not captured in our data collection; (4) the effect of the velocity distribution at the upstream boundary of the site²⁷; and (5) the measured velocities being the component of the velocity in the downstream direction, while the velocities predicted by the 2-D model were the absolute magnitude of the velocity²⁸. As shown in the figures in Appendix C, we attribute most of the differences between measured and predicted velocities to noise in the measured velocity measurements; specifically, for the transects, the simulated velocities typically fell within the range of the measured velocities of the two to three ADCP traverses made on each transect. The 2-D model integrates effects from the surrounding elements at each point. Thus, point measurements of velocity can differ from simulated values simply due to the local area integration that takes place. As a result, the area integration effect noted above will produce somewhat smoother lateral velocity profiles than the observations.

The flow and downstream WSEL in the calibrated Bridge Riffle site cdg file were changed to simulate the hydrodynamics of the sites at the simulation flows (3,250 cfs to 5,500 cfs by 250 cfs increments, 5,500 cfs to 8,000 cfs by 500 cfs increments, 8,000 cfs to 15,000 cfs by 1,000 cfs increments, and 15,000 cfs to 31,000 cfs by 2,000 cfs increments). The cdg file for each flow contained the WSEL predicted by PHABSIM at the downstream transect at that flow minus 0.1

Velocities were plotted versus northing since the transects were orientated primarily north-south.

River2D distributes velocities across the upstream boundary in proportion to depth, so that the fastest velocities are at the thalweg. In contrast, the bed topography of a site may be such that the fastest measured velocities may be located in a different part of the channel. Since we did not measure the bed topography above a site, this may result in River2D improperly distributing the flow across the top of the site. As discussed above, we added artificial upstream extensions to the sites to try to address this issue.

For areas with transverse flow, this would result in the 2-D model appearing to overpredict velocities even if it was actually accurately predicting the velocities.

foot. Each discharge was run in RIVER2D to steady state. Again, a stable solution will generally have a Sol Δ of less than 0.00001 and a Net Q of less than 1%. In addition, solutions will usually have a Max F of less than one. The production cdg files all had a Sol Δ of less than 0.00001 and a Net Q of less than 1% (Table 8). The maximum Froude Number was greater than 1 for 16 simulated flows (Table 8); however, we considered these production runs to be acceptable since the Froude Number was only greater than 1 at a few nodes, with the vast majority of the area within the site having Froude Numbers less than 1. Also, these nodes were located either at water's edge or where water depth was extremely shallow, typically approaching zero and would be expected to have an insignificant effect on the model results.

Habitat Suitability Criteria (HSC) Development

We assumed that there would be reduced survival of eggs or pre-emergent fry, and thus spawning habitat would be lost, if the tailspill was exposed or if velocities dropped to the point where there was insufficient intragravel flow through the redd. While we did not make measurements on tailspill depths for Sacramento River redds, we did take these measurements for 851 fall-run chinook salmon and 106 steelhead/rainbow trout redds on the Yuba River. There was a significant positive correlation between the depth of the redds and the difference between the redd depth and tailspill depth for both fall-run chinook salmon ($R^2 = 0.74$, $p = 2 \times 10^{-251}$) and steelhead/rainbow trout ($R^2 = 0.04$, p = 0.03) redds (Figures 3 and 4). When only redds with depths less than 2 feet²⁹ were considered, the correlations for fall-run chinook salmon (n = 664, $R^2 = 0.31$, $p = 1 \times 10^{-55}$) and steelhead/rainbow trout (n = 26, $R^2 = 0.39$, p = 0.0005) were still significant. However, since we needed to pick a single value of the difference between the tailspill and redd depths for the redd dewatering analysis, we selected the average difference for fall-run chinook salmon (0.5 foot) and steelhead/rainbow trout (0.2 foot) redds with redd depths less than 2 feet. If the tailspill is 0.5 foot higher than the depth at the head of the pit (the depth used to compute spawning habitat), chinook salmon spawning habitat would be lost if the spawning depth fell below 0.5 foot. Similarly, if the tailspill is 0.2 foot higher than the depth at the head of the pit (the depth used to compute spawning habitat), steelhead spawning habitat would be lost if the spawning depth fell below 0.2 foot. We assumed that there would be insufficient intragravel flow through the redd if the spawning velocity was less than the lowest velocity at which we found a fall-run, late-fall-run or winter-run chinook salmon redd in the Sacramento River, or the lowest velocity of a steelhead redd in the Lower American River (the source of the steelhead spawning HSC used for the Sacramento River). The lowest velocities we found in measurements of Sacramento River fall-run, late-fall-run and winter-run chinook salmon were, respectively, 0.32 ft/s, 0.32 ft/s and 0.87 ft/s (U.S. Fish and Wildlife Service 2003). The lowest velocity of a steelhead redd in the Lower American River was 0.73 ft/s (U.S. Fish and Wildlife Service 1996a). The redd dewatering criteria used are given in Table 9.

²⁹ Two feet was selected because the drop in stage associated with a change in flow for the Sacramento River sites is typically less than two feet.

Table 8
Bridge Riffle Site Simulation Statistics

Flow (cfs)	Net Q	Sol A	Max F
3250	0.3%	.000004	1.34
3500	0.3%	.000008	1.35
3750	0.3%	.000007	1.36
4000	0.2%	.000004	1.38
4250	0.1%	.000005	1.41
4500	0.1%	.000005	1.46
4750	0.02%	<.00001	1.46
5000	0.04%	.000003	1.45
5250	0.1%	.000008	1.43
5500	0.1%	.000002	1.33
6000	0.01%	.000003	1.05
6500	0.04%	.000006	1.00
7000	0.2%	.000005	1.55
7500	0.2%	.000004	1.00
8000	0.3%	.000003	1.00
9000	0.7%	<.000001	1.00
10000	0.01%	<.000001	1.00
11000	0.1%	<.000001	1.03
12000	0.1%	<.000001	1.00
13000	0.1%	<.000001	1.00
14000	0.6%	.000003	1.00
15000	0.2%	<.000001	1.00
17000	0.4%	<.000001	1.00
19000	0.6%	<.000001	1.00
21000	0.8%	.000006	1.00
23000	0.5%	.000003	1.00
25000	0.04%	<.000001	1.38
27000	0.1%	<.000001	1.06
29000	0.05%	<.00001	1.00
31000	0.1%	<.00001	4.34

Figure 3
Tailspill and Redd Depths for Yuba River Fall-run Chinook Salmon Redds

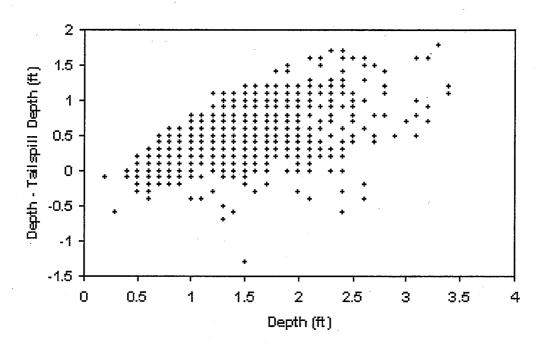


Figure 4
Tailspill and Redd Depths for Yuba River Steelhead/Rainbow Trout Redds

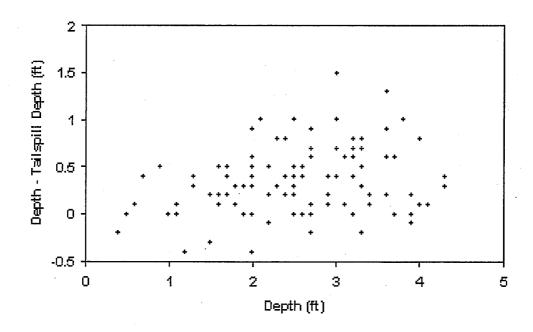


Table 9
Redd Dewatering HSC

Water		Water		Channel	
Velocity (ft/s)	SI Value	Depth (ft)	SI Value	Index Value	SI Value
Fall-run Chinook Salmon					
0.00	0.00	0.00	0.00	0.00	0.00
0.31	0.00	0.50	0.00	1.00	1.00
0.32	1.00	0.52	1.00	100.0	1.00
100.0	1.00	100.0	1.00		
Late-fall-run Chinook Salmon					
0.00	0.00	0.00	0.00	0.00	0.00
0.31	0.00	0.50	0.00	1.00	1.00
0.32	1.00	0.52	1.00	100.0	1.00
100.0	1.00	100.0	1.00		,
	•	Winter-run Ch	inook Salmon		
0.00	0.00	0.00	0.00	0.00	0.00
0.86	0.00	0.50	0.00	1.00	1.00
0.87	1.00	0.52	1.00	100.0	1.00
100.0	1.00	100.0	1.00		
Steelhead					
0.00	0.00	0.00	0.00	0.00	0.00
0.72	0.00	0.20	0.00	1.00	1.00
0.73	1.00	0.23	1.00	100.0	1.00
100.0	1.00	100.0	1.00		

Habitat Simulation

We classified the stranding sites as either off-channel-areas or in-channel-areas (Appendix A). In our 1996 snorkel surveys of juvenile chinook salmon (U.S. Fish and Wildlife Service 1996b), we found an average of 6.3 fish/1000 ft² in off-channel-areas, and an average of 30.2 fish/1000 ft² in in-channel-areas. We multiplied the areas of each stranding site by the appropriate fish density to determine the average number of fish in each stranding site. These were summed by range of stranding flows to determine the total number of juvenile salmon stranded in the Sacramento River between Keswick Dam and Battle Creek for different drops in flow (Appendix D). For example, if the Sacramento River flow drops from 3,750 to 3,250 cfs, 11,227 fish will be stranded³0. In contrast, if the Sacramento River flow drops from 15,000 to 4,000 cfs with the ACID dam boards in, 12,044 fish will be stranded. There are minor differences in the number of fish stranded with the ACID dam boards out versus in due to the different stranding flows for stranding sites 1 to 8 when the ACID dam boards are out versus in.

We conducted an effective spawning analysis with River2D to determine the percentage loss of fall-run, late-fall-run and winter-run chinook salmon and steelhead spawning habitat in the Sacramento River between Keswick Dam and Battle Creek associated with drops in flow. An effective spawning analysis examines on a node by node basis the depths and velocities at lower flows, and sets the weighted useable area represented by each node at a given flow to zero if the depth or velocity at a lower flow are less than the parameters in Table 9; if the depth and velocity at the lower flow are both greater than the parameters in Table 9, the weighted useable area represented by a given node is not changed. By adding up the resulting weighted useable areas represented by all the nodes, the effective spawning analysis computes how much weighted useable area remains after the flow drops. The percentage loss in spawning habitat is then computed as:

(original spawning habitat - remaining spawning habitat)/original spawning habitat

We conducted the effective spawning habitat analysis by producing an output file containing the spawning combined habitat suitability from River2D with the spawning flow file for a given site. This file was then used as a channel index file for the River2D files for the dewatering flows for that site, along with the HSC in Table 9, to compute the remaining spawning habitat.

A byproduct of the effective spawning analysis were new flow-habitat relationships for fall-run, late-fall-run and winter-run chinook salmon and steelhead spawning for the eight spawning sites (Appendix F) and the three segments between Keswick Dam and Battle Creek (Appendix G) computed using River2D.

³⁰ 11,227 fish is the total number of juvenile fish for the stranding sites (sites 36, 45B, 51, 51A, 61A and 71) that strand between 3,750 and 3,250 cfs. In the context of this report, juvenile fish refer to any young-of-the-year salmon, generally in the size range of 35 to 100 mm.

RESULTS

The effects of flow drops on juvenile salmon entrapment stranding are shown in Figures 5 and 6 and Appendix D. The results indicate that, as expected, greater drops in flow are associated with increased numbers of stranded juvenile salmon, but that substantial juvenile stranding could be avoided by keeping flows above 3,750 cfs. These results could be used to determine the amount of take of juvenile salmon associated with a given drop in releases of flow from Keswick Dam. The effects of flow drops on redd dewatering are shown in Figures 7 to 14 and Appendix E. Similar to juvenile stranding, the results of the redd dewatering analysis indicate that, as expected, greater drops in flow result in a greater percentage of salmon and steelhead redds being dewatered. These results also suggest that the percentage of redds dewatered associated with a given drop of flow is less with the ACID Dam in, versus the ACID Dam out, reflecting the deeper water conditions above the ACID Dam when the dam is in versus when the dam is out. These results could be used to determine the amount of take of chinook salmon and steelhead eggs and pre-emergent fry associated with a given drop in releases of flow from Keswick Dam.

Flow-habitat relationships for fall-run, late-fall-run and winter-run chinook salmon and steelhead spawning, calculated using River2D, are given in Appendices F and G. We recommend that these results be used, rather than the results in U.S. Fish and Wildlife Service 2003, because of the improved prediction of flow-habitat relationships with River2D, versus PHABSIM.

Figure 5
Stranding of Juvenile Chinook Salmon with ACID Dam out

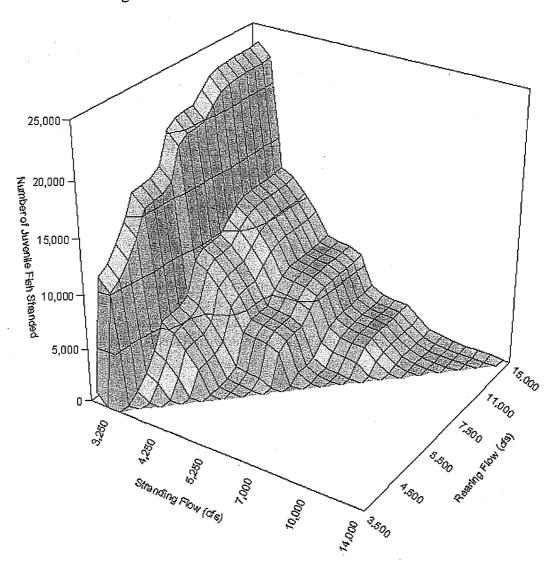


Figure 6
Stranding of Juvenile Chinook Salmon with ACID Dam in

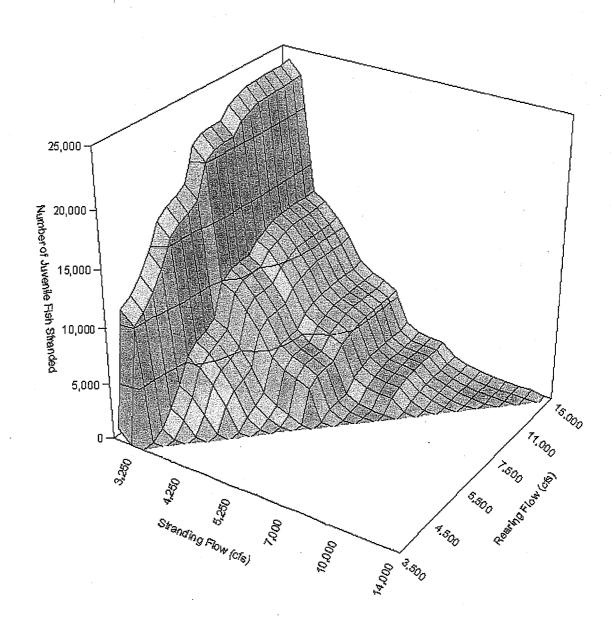


Figure 7
Dewatering of Fall-run Chinook Salmon Redds with ACID Dam out

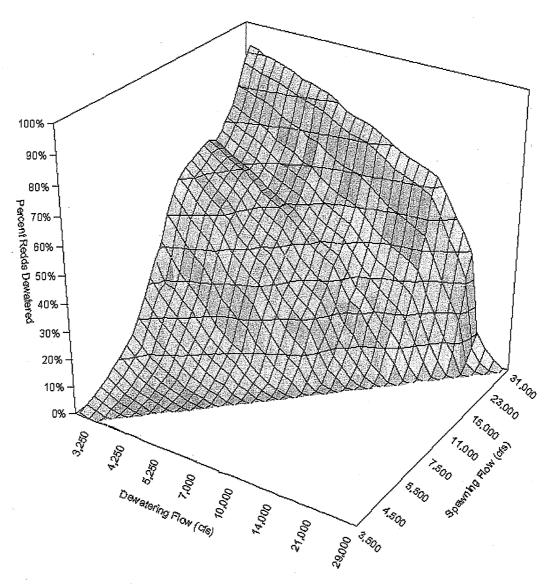


Figure 8
Dewatering of Fall-run Chinook Salmon Redds with ACID Dam in

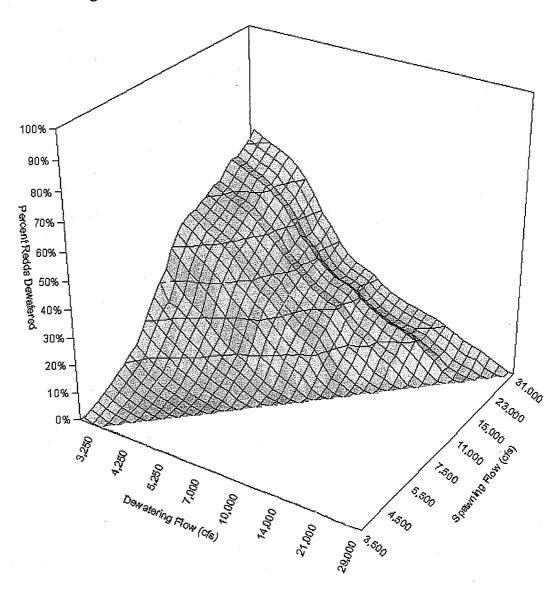


Figure 9
Dewatering of Late-Fall-run Chinook Salmon Redds with ACID Dam out

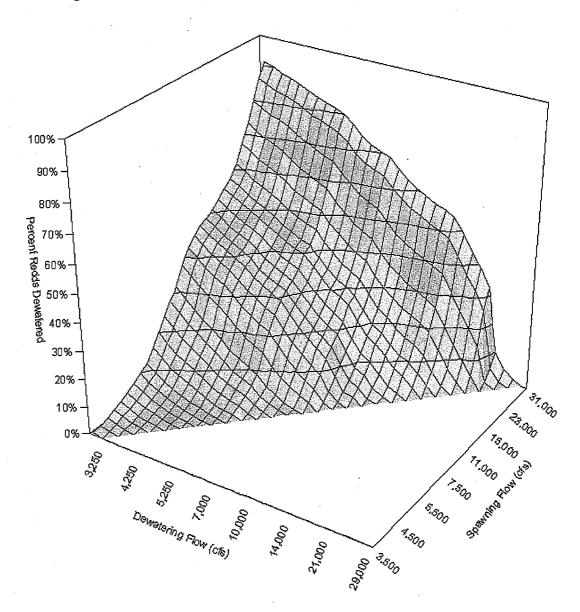


Figure 10
Dewatering of Late-Fall-run Chinook Salmon Redds with ACID Dam in

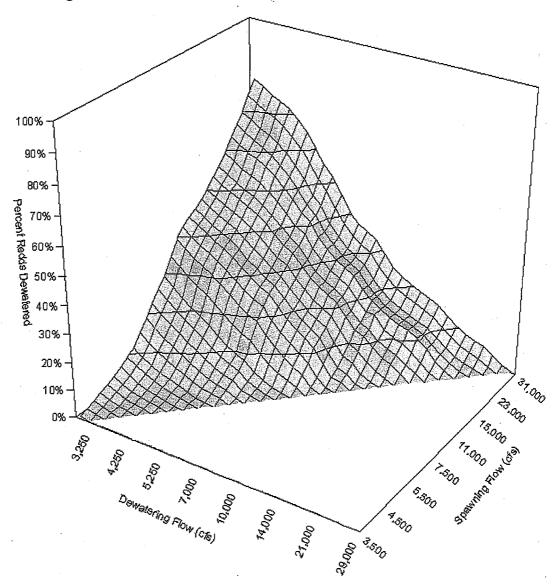


Figure 11
Dewatering of Winter-run Chinook Salmon Redds with ACID Dam out

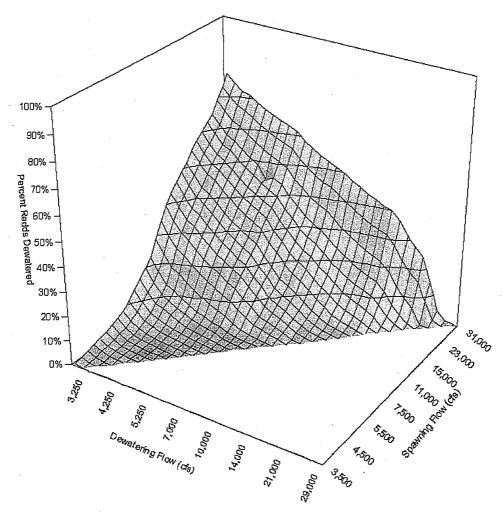


Figure 12
Dewatering of Winter-run Chinook Salmon Redds with ACID Dam in

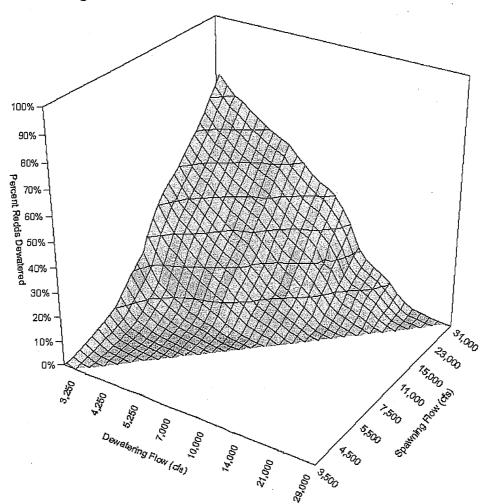


Figure 13
Dewatering of Steelhead Redds with ACID Dam out

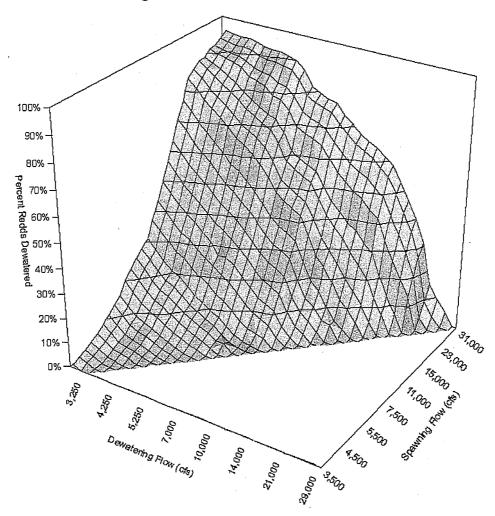
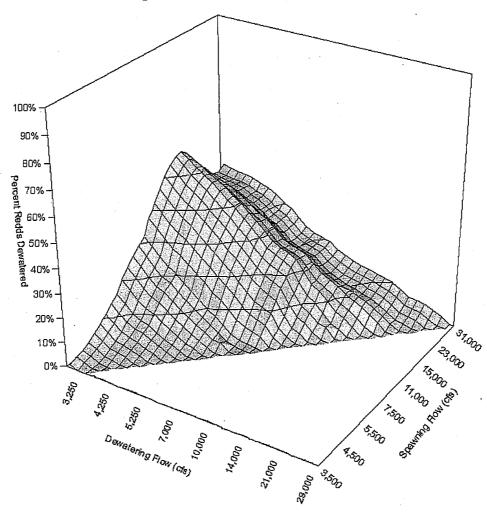


Figure 14
Dewatering of Steelhead Redds with ACID Dam in



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APPENDIX A STRANDING SITE LOCATIONS

Stranding Site #	River Mile	River Bank	MHU#	Stranding Flow (cfs) ³¹	Stranding Area (ft²)	Type
1	298.8	Left	139	21,250/5,000	19,579	In
3A	300.6	Left	143	12,750/11,100	684	OCA
3B	300.6	Left	143	5,200/4,625	2,673	OCA
4	300.8	Left	143	7,400/6,580	4,838	OCA
. 5	301.4	Left	143	20,000/4,825	2,107	OCA
6	302.0	Right	143	8,128	1,200	In
7	300.2	Right	141	5,250/<3,250	2,850	In
8	299.9	Right	140	8,200/5,100	12,906	In
9	292.5	Left	100	6,409	1,319	OCA
10	294.0	Left	109	5,950	600	OCA
11	295.2	Left	113	<3,250		OCA
12	295.2	Left	113	<3,250	8,303	OCA
13	296.4	Left	129	4,500	1,056	OCA
14	296.5	Left	127	4,555	200,000	OCA
15	297.0	Left	127	<3,250	5,373	OCA
16	297.4	Left	133	<3,250	,75,024	In
17	296.9	Right	132	4,844	1,296	OCA
18	296.7	Right	130	9,376	387	OCA
19	296.3	Right	123	5,950	3,164	In
20	295.5	N/A	114	9,337	13,640	OCA
21	295.3	N/A	114	6,050	47,611	OCA
22	294.9	Right	111	<3,250	594	OCA
23	291.7	Right	96	4,360	4,497	In
24	291.8	Left	97	6,032	2,640	In
25	291.8	Right	97	4,248	5,612	In

Sites 1 to 5, 7 and 8 are located above ACID and have a different stranding flow depending on whether the boards are in or out at ACID. The first flow is the stranding flow with boards out, while the second flow is the stranding flow with boards out.

Stranding Site #	River Mile	River Bank	MHU#	Stranding Flow (cfs)	Stranding Area (ft²)	Type
26	289.5	Right	80	4,849	423	OCA
27	293.7	N/A	107	3,946	106,000	OCA
28	293.7	Right	109	<3,250	1,352	OCA
29	293.7	Right	108	7,483	300	OCA
30	293.1	Right	104	5,921	26,978	In
31	292.8	Right	·104	14,276	580	OCA
32	292.8	Right	104	7,683	26,371	In
33	291.5	Right	91	14,927	21,500	OCA
34	290.3	Right	85	5,934	11,606	OCA
35	289.3	Middle	75	7,898	4,397	In
36	289.3	Middle	75	3,450	36,320	In
37	288.5	Right	67	<3,250	4,700	OCA
38	288.5	Right	67	13,771	429	OCA
39A	291.7	Left	98	4,752	4,118	OCA
39B	291.7	Left	98	10,508	533	OCA
40	291.4	Left	91	10,747	13,739	OCA
41	290.3	Left	85	7,330	5,921	OCA
41A	290.3	Left	85	4,640	3,233	In
42	290.3	Left	85	7,683>Q>4,710	3,050	OCA
43	290.3	Left	85	4,440	9,020	OCA
44	290.0	Left .	85	9,514	18,631	OCA
45A	290.0	N/A	84	<3,250	2,649	In
45B	290.0	N/A	84	3,502	87,352	In
46	289.8	N/A	83	4,108	34,126	In
47	289.5	Left	81	9,661	432	OCA
48	289.4	Left	75	8,277	333	In
49	289.8	Left	83	4,640	5,066	In
50	289.6	N/A	82	4,440	40,594	In

Stranding Site #	River Mile	River Bank	<u>MHU #</u>	Stranding Flow (cfs)	Stranding Area (ft²)	Type
51	289.5	N/A	78, 80, 82	3,502	222,752	In
51A	289.5	N/A	79	3,502	64,688	OCA
52	289.4	N/A	76	6,180	3,827	In
53	289.4	N/A	76	4,666	17,375	In
54	289.4	N/A	76	4,766	4,261	In
55	289.8	Right	84	14,727	3,630	OCA
56	289.7	Right	84	4,440	2,088	In
57	285.2	Left	46	5,265	713	In
58	283.3	Left	45	<3,250	771	OCA
59	284.9	Left	46	6,086	760	In
60	287.7	N/A	61	<3,250		In
60A	287.7	Right	61	8,762	1,330	In
60B	287.7	Right	61	8,962	1,170	In
61	287.9	Left	63	5,752	30,437	In
61A	287.9	Left	63	3,568	11,727	In
61B	287.9	Left	63	6,286	624	In
62	287.8	N/A	61	<3,250		In
63	287.9	Right	64	8,762	480	In
64	287.6	Right	59	8,562	583	OCA
65	287.5	Right	60	8,762	943	OCA
66	286.3	Right	53	10,859	3,049	OCA
67	286.3	Right	53	5,986	924	In
68	285.4	Right	48	5,460	84,638	OCA
69	285.2	Right	47	4,450	2,345	OCA
70	285.2	Right	47	5,100	2,669	OCA
71	284.3	Right	45	3,664	493	OCA
72	283.6	Right	45	12,643	722	OCA

Stranding Site #	River Mile	River Bank	<u>MHU #</u>	Stranding Flow (cfs)	Stranding Area (ft²)	Type
73	282.8	Right	43	5,750	364	OCA
74	282.6	Right	42	4,591	235	OCA
75	281.3	Right	36	<3,250	42,066	OCA
76	281.3	Right	36	8,826	5,918	In
77	281.0	Right	34	6,744	2,341	OCA
78	280.6	Right	33	6,672	2,331	OCA
79B	280.6	Right	33	8,364	120	OCA
79C	280.6	Right	33	8,926	1,691	OCA
79A	280.4	Left	31	8,926	693	In
80	279.9	Right	28	9,430	459	In
81	279.1	Right	26	13,546	1,814	OCA
82	273.0	Right	9	18,799	702	OCA
83	283.1	Left	44	< 3250	675	OCA
84	282.7	Left	43	9,100	3451	OCA
85.	282.6	Left	41	28,422	7,097	OCA
86	280.8	Right	34	6,542	2,153	In
87	280.4	Right	30	6,417	2,129	In
88	280.3	Right	30	8,287	1,746	In
88A	280.3	Right	30	11,496	1,089	In
89	280.3	Right	30	7,937	50	OCA
90	280.2	Left	30	5,674	650	OCA
91	278.5	Left	20	9,333	3,683	OCA
92	276.9	Left	14	8,333	1,871	OCA
93	275.6	Left	12	15,071	738	OCA
94	275.6	Left	12	11,083	675	OCA
95	271.7	Right	6	5,542	27,003	In
96	287.6	Right	21	9,406	1,159	In
97	287.6	Right	21	9,568	564	In

APPENDIX B PHABSIM WSEL CALIBRATION

Stage of Zero Flow Values

Stranding Site	SZF
6	61.3
9	85.2
11	85.7
12	89.5
13	80.8
14	92.7
15	95.8
16	81.6
17	87.7
20	86.8
22	89.2
24, 25	92.8
27, 28	89.4
30	89.1
31	86.8
34	85.3
37, 38	76.5
39	92.1
40	93.0
44, 45	88.5
46	91.8
48	87.1
57	79.8
58	86.6

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Stranding Site	SZF
61	90.3
66	96.0
68	88.1
69, 70	78.0
71	88.7
72	87.3
74	89.3
77	88.5
78, 79B	80.2
81	90.7
85	90.4
86	87.0
87	95.0
88	94.3
88A	95.8
89	93.6
90	90.5
94	83.8
97	88.7

SITE	BETA COEFF.	%MEAN ERROR	Calculated 6580 cfs	vs. Given D 10045 cfs	isch. (%) 12032 cfs	Difference (mo	easured vs. pr 10045 cfs	ed. WSELs) 12032 cfs
6	3.88	0.3	0.2	0.5	0.4	0.01	0.04	0.03
SITE	BETA COEFF.	%MEAN ERROR	Calculated 6152 cfs	vs. Given D <u>9813 cfs</u>	isch. (%) <u>11822 cfs</u>	Difference (me	easured vs. pr 9813 cfs	ed. WSELs) 11822 cfs
9	3.78	0.1	0.0	0.2	0.1	0.00 .	0.01	0.00
11	5.56	0.6	0.3	0.9	0.4	0.01	0.02	0.01
12	3.79	0.4	0.2	0.6	0.5	0.00	0.01	0.01
13	7.05	0.4	0.2	0.6	0.4	0.00	0.01	0.01
16	4.02	3.4	2.1	5.3	3.0	0.07	0.18	0.11
17	2.91	0.3	0.1	0.5	0.3	0.00	0.02	0.01
20	3.01	0.0	0.0	0.0	0.0	0.00	0.00	0.00
22	2.73	1.0	0.5	1.5	1.0	0.01	0.05	0.04
	DETA	OZNATE ANI	Calaulata d	Cioren D	inal (0/)	Difference (m)		od WCEI a)
SITE	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	6152 cfs	vs. Given D 9812 cfs	11822 cfs	Difference (me	9812 cfs	11822 cfs
14	2.72	1.9	0.9	3.0	2.0	0.01	0.05	0.04
<u>SITE</u>	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	Calculated 7910 cfs	vs. Given D 11700 cfs	isch. (%) 14990 cfs	Difference (me	easured vs. pr 11700 cfs	ed. WSELs) 14990 cfs
15	1.96	3.2	2.2	4.9	2.6	0.04	0.11	0.07
SITE	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	Calculated 6092 cfs	vs. Given D <u>9808 cfs</u>	isch. (%) 2418 cfs	Difference (me	easured vs. pr 9808 cfs	ed. WSELs) 2418 cfs
24, 25	2.07	0.9	0.3	1.3	1.0	0.00	0.02	0.02
		4				•		
SITE	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 7683 cfs	vs. Given D <u>11700 cfs</u>	isch. (%) <u>14998 cfs</u>	Difference (me	easured vs. pr 11700 cfs	ed. WSELs) 14998 cfs
27, 28	3.36	0.5	0.3	0.8	0.5	0.01	0.02	0.01

<u>SITE</u>	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 7683 cfs	vs. Given D 11700 cfs	isch. (%) <u>14987 cfs</u>	Difference (me	easured vs. pr 11700 cfs	ed. WSELs) 14987 cfs
31	2.57	0.8	0.5	1.3	0.8	0.01	0.04	0.03
34	2.91	0.0	0.0	0.0	0.0	0.00	0.00	0.00
<u>SITE</u>	BETA COEFF.	%MEAN ERROR	Calculated 4710 cfs	vs. Given D 8608 cfs	isch. (%) 14987 cfs	Difference (me	easured vs. pr 8608 cfs	ed. WSELs) 14987 cfs
30	2.11	3.4	2.1	5.0	3.1	0.06	0.19	0.14
<u>SITE</u>	BETA COEFF.	%MEAN ERROR	7898 cfs		14986 cfs	Difference (me	12009 cfs	14986 cfs
37, 38	6.05	0.1	0.0	0.1	0.1	0.00	0.00	0.00
<u>SITE</u>	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 7554 cfs	vs. Given D 11700 cfs	isch. (%) 14927 cfs	Difference (me	easured vs. pr 11700 cfs	ed. WSELs) 14927 cfs
39A, B	2.16	2.4	1.5	3.6	2.0	0.04	0.11	0.07
<u>SITE</u>	BETA COEFF.	%MEAN ERROR	6150 cfs		14927 cfs	Difference (me	7554 cfs	14927 cfs
40	2.30	0.5	0.6	0.8	0.2	0.01	0.02	0.01
<u>SITE</u>	BETA COEFF.	%MEAN ERROR	Calculated 4710 cfs	vs. Given D 7554 cfs	isch. (%) 11700 cfs	Difference (mo	easured vs. pr 7554 cfs	ed. WSELs) 11700 cfs
44; 45A, B	2.99	2.6	1.6	3.8	2.3	0.03	0.09	0.06
46	2.62	3.2	1.9	4.7	2.9	0.04	0.11	0.08
<u>SITE</u>	BETA COEFF.	%MEAN ERROR	Calculated 7554 cfs	vs. Given D 11700 cfs	isch. (%) <u>14927 cfs</u>	Difference (mo	easured vs. pr 11700 cfs	ed. WSELs) 14927 cfs
48	4.25	1.3	0.8	2.1	1.2	0.02	0.05	. 0.05
SITE	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 4936 cfs	vs. Given D <u>6068 cfs</u>	isch. (%)	Difference (mo	easured vs. pr 6068 cfs	red. WSELs)
57	1.26					0.00	0.00	

SITE	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 6244 cfs		isch. (%) 14988 cfs	Difference (me	asured vs. pi 8994 cfs	red. WSELs) 14988 cfs
58	3.24	2.0	1.9	3.1	1.1	0.05	0.09	0.04
<u>SITE</u>	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	Calculated 6106 cfs	vs. Given D <u>8762 cfs</u>	isch. (%) <u>14986 cfs</u>	Difference (me 6106 cfs	asured vs. pi 8762 cfs	red. WSELs) 14986 cfs
61, 61A	4.04	1.0	0.9	1.5	0.6	0.01	0.02	0.01
SITE	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 6086 cfs	vs. Given D 8700 cfs	isch. (%) 14986 cfs	Difference (me 6086 cfs	asured vs. pi 8700 cfs	red. WSELs) 14986 cfs
68	2.87	0.5	0.5	0.8	0.3	0.01	0.03	0.01
69, 70	6.17	2.0	1.9	3.0	1.0	0.06 '	0.10	0.04
SITE	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	Calculated 6244 cfs	vs. Given D 8926 cfs	isch. (%) 14986 cfs	Difference (me <u>6244 cfs</u>	asured vs. pr 8926 cfs	ed. WSELs) 14986 cfs
71	3.01	2.1	2.0	3.2	1.1	0.05	0.09	0.04
						7.100		1
SITE	BETA COEFF.	%MEAN <u>ERROR</u>	Calculated 6244 cfs	vs. Given D 8926 cfs	isch. (%) 14988 cfs	Difference (me <u>6244 cfs</u>	asured vs. pr 8926 cfs	ed. WSELs) 14988 cfs
72	1.88	2.2	2.2	3.4	1.2	0.04	0.09	0.04
74	1.83	1.1	1.1	1.7	0.7	0.03	0.05	0.03
77	2.42	2.9	2.9	4.4	1.4	0.09	0.13	0.06
78, 79B	4.33	0.4	0.4	0.7	0.3	0.01	0.02	0.01
SITE	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	Calculated 6454 cfs	vs. Given D 9428 cfs	isch. (%) 15049 cfs	Difference (me 6454 cfs	asured vs. pr 9428 cfs	red. WSELs) 15049 cfs
81	1.97	1.1	0.9	1.7	0.8	0.02	0.06	0.03
CITE	BETA	%MEAN	Calculated 6244 cfs	vs. Given D <u>9035 cfs</u>	isch. (%) <u>14580 cfs</u>	Difference (me 6244 cfs	asured vs. pr 9035 cfs	red. WSELs) 14580 cfs
SITE	COEFF.	ERROR				,		
85	3.16	2.7	2.6	4.2	1.5	0.04	0.07	0.03
	BETA	%MEAN	Calculated	vs. Given D	isch. (%)	Difference (me	asured vs. pr	ed. WSELs)
SITE	COEFF.	<u>ERROR</u>	6244 cfs	9035 cfs	14988 cfs	6244 cfs	9035 cfs	14988 cfs
86	3.50	2.2	2.1	3.3	1.2	0.06	0.10	0.04

SITE	BETA <u>COEFF.</u>	%MEAN <u>ERROR</u>	Calculated 5977 cfs	vs. Given D <u>10109 cfs</u>	isch. (%)	Difference (m 5977 cfs	easured vs. pr 10109 cfs	red. WSELs)
87	. 9.22					0.00	0.00	
90	3.26					0.00	0.00	
2								
SITE	BETA COEFF.	%MEAN ERROR	Calculated 6106 cfs		isch. (%) 14988 cfs	Difference (median 6106 cfs	easured vs. pr 10109 cfs	ed. WSELs) 14988 cfs
88	4.83	4.1	1.8	6.4	4.3	0.01	0.05	0.04
89	2.36	4.6	2.0	7.2	4.8	0.01	0.10	0.10
SITE	BETA COEFF.	%MEAN ERROR	Calculated 10109 cfs	vs. Given D 14988 cfs	isch. (%)	Difference (me	easured vs. pr 14988 cfs	ed. WSELs)
<u>SITE</u> 88A					isch. (%)	•	*	ed. WSELs)
88A	COEFF. 1.77 BETA	ERROR %MEAN	10109 cfs Calculated	14988 cfs vs. Given D	isch. (%)	10109 cfs 0.00 Difference (me	14988 cfs 0.00 easured vs. pr	ed. WSELs)
	<u>COEFF.</u> 1.77	ERROR 	10109 cfs	14988 cfs	, ,	10109 cfs 0.00	14988 cfs 0.00	·
88A	COEFF. 1.77 BETA	ERROR %MEAN	10109 cfs Calculated	14988 cfs vs. Given D	isch. (%)	10109 cfs 0.00 Difference (me	14988 cfs 0.00 easured vs. pr	ed. WSELs)
88A SITE	COEFF. 1.77 BETA COEFF.	ERROR %MEAN ERROR	10109 cfs Calculated 6511 cfs	14988 cfs vs. Given D 9606 cfs	isch. (%) 15071 cfs	10109 cfs 0.00 Difference (me 6511 cfs	14988 cfs 0.00 easured vs. pr 9606 cfs	ed. WSELs) 15071 cfs
88A SITE	COEFF. 1.77 BETA COEFF.	ERROR %MEAN ERROR	10109 cfs Calculated 6511 cfs	14988 cfs vs. Given D 9606 cfs 1.0 vs. Given D	isch. (%) 15071 cfs 0.4	10109 cfs 0.00 Difference (me 6511 cfs	14988 cfs 0.00 easured vs. pr 9606 cfs 0.03	ed. WSELs) 15071 cfs 0.02

APPENDIX C BRIDGE RIFFLE VELOCITY VALIDATION STATISTICS

Measured Velocities less than 3 ft/s

Difference (measured vs. pred. velocities, ft/s)

Number of Observations	Average	Standard Deviation	Maximum
87	1.72	1.67	6.15

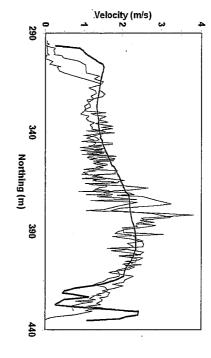
Measured Velocities greater than 3 ft/s

Percent Difference (measured vs. pred. velocities)

Number of Observations	Average	Standard Deviation	Maximum
304	30%	25%	107%

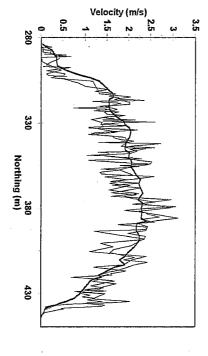
All differences were calculated as the absolute value of the difference between the measured and simulated velocity.

Bridge Riffle XS1, Q = 14454 cfs



2-D Simulated Velocities —— Measured Velocities

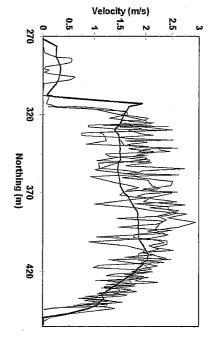
Bridge Riffle XS2, Q = 15149 cfs



2-D Simulated Velocities — Measured Velocities

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Bridge Riffle XS3, Q = 15149 cfs



---- 2-D Simulated Vetocities ---- Measured Velocities

APPENDIX D JUVENILE STRANDING RESULTS

	0059											629	3385	3940	4060	4225	6262	7765	8965	9633	19763	20860
	0009												2775	3330	3450	3615	5652	7155	8355	9023	19153	20250
	2200													574	694	829	2896	4399	5599	6267	16397	17494
ards Out	5250														139	304	2341	3844	5044	5712	15842	16939
Dam Bo	2000	fs)														185	2222	3725	4925	5592	15722	16819
Number of juvenile salmon stranded - ACID Dam Boards Out	4750	Rearing flow (cfs)															2056	3559	4759	5427	15557	16654
on strand	4500	Rea																1503	2703	3371	13501	14598
nile salm	4250																		1200	1868	11998	13095
er of juve	4000														•					899	10798	11895
Numb	3750										٠			٠.							10130	11227
ν.	3500				•																	1097
		14000	13000	12000	11000	10000	0006	8000	7500	7000	0039	0009	2200	5250	2000	4750	4500	4250	4000	3750	3500	3250

Stranding flow (cfs)

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Number of juvenile salmon stranded - ACID Dam Boards Out (continued) Rearing flow (cfs)

Stranding flow (cfs)

	0099											629	3385	3940	4346	5116	7170	8673	9873	10541	20671
	0009												2775	3330	3737	4507	0959	8063	9264	9931	20061
	2200													574	981	1751	3804	5307	6507	7175	17305
Soards In	5250								٠						426	1196	3249	4753	5953	6620	16750
ID Dam I	2000	ofs)														789	2843	4346	5546	6214	16344
Number of juvenile salmon stranded - ACID Dam Boards In	4750	Rearing flow (cfs)				-											2073	3576	4776	5444	15574
non stran	4500	Reg																1503	2703	3371	13501
renile salı	4250														•				1200	1868	11998
ber of juv	4000																			899	10798
Num	3750																				10130
	3500																				
		14000	13000	12000	11000	10000	0006	8000	7500	7000	6500	0009	2200	5250	2000	4750	4500	4250	4000	3750	3500

Stranding flow (cfs)

USFWS, SFWO, Energy Planning and Instream Flow Branch Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

USFWS, SFWO, Energy Planning and Instream Flow Branch
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							;	Stra	ndin	g flo	w (c	fs)										
3250	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	10000	11000	12000	13000	14000		
21893	20796	10666	9998	8798	7295	5241	4471	4065	3510	754	144										7000	Number
21932	20835	10705	10037	8837	7334	5281	4510	4104	3549	793	183	58	,								7500	of juveni
22861	21765	11635	10967	9767	8264	6210	5440	5033	4479	1723	1113	988	949								8000	Number of juvenile salmon stranded - ACID
23283	22186	12056	11388	10188	8685	6631	5861	5455	4900	2144	1534	1409	1370	421						Rea	9000	stranded
23602	22505	12375	11708	10508	9004	6951	6181	5774	5219	2463	1854	1729	1690	741	319					Rearing flow (cfs)	10000	- ACID I
23711	22614	12485	11817	10617	9114	7060	6290	5883	5329	2572	1963	1838	1799	850	428	109				ofs)	11000	Dam Boards In (continued)
23753	22656	12526	11858	10658	9155	7101	6331	5925	5370	2614	2004	1879	1840	891	470	151	41				12000	ds In (con
23757	22660	12531	11863	10663	9160	7106	6336	5929	5375	2618	2009	1884	1845	896	474	155	46	ΟΊ			13000	tinued)
23771	22675	12545	11877	10677	9174	7120	6350	5943	5389	2633	2023	1898	1859	910	489	169	60	19	14		14000	
23938	22841	12711	12044	10843	9340	7287	6517	6110	5555	2799	2190	2065	2025	1077	655	336	227	185	181	167	15000	

APPENDIX E REDD DEWATERING RESULTS

ls Out
n Board
) Dam
- ACID
Dewatered
Redds]
Salmon
Chinook
f Fall-run
Percentage of

3500																								7.
3750																							1.0%	1.0% 2.0%
_							•											,				0.9%	, 2.1%	6 3.4%
4000 4250																					0.9%	1.6%	3.2%	4.8%
					٠															0.8%	1.7%	2.6%	4.6%	%9:9
4500 4750 5000 5250 5500 6000 6500 7000 7500 8000 9	٠																		0.8%	1.6%	2.8%	3.9%	6.2%	8.4%
5000 5000	Spaw																٠	0.8%	1.7%	2.7%	4.1%	5.5%	8.1%	10.6%
5250	Spawning Flow (cfs)																0.7%	1.6%	2.8%	4.0%	2.7%	7.3%	10.1%	12.9%
5500	(cfs)															0.7%	1.3%	2.5%	4.0%	5.4%	7.3%	9.2%	12.2%	15.3%
6000															1.4%	2.1%	3.2%	4.8%	6.9%	8.9%	11.4%	13.6%	17.0%	20.6%
6500														1.2%	3.2%	4.2%	2.6%	7.6%	10.4%	13.0%	15.8%	18.4%	22.2%	26.2%
7000						,							1.3%	2.8%	5.4%	8.9%	8.6%	10.8%	14.2%	17.2%	20.3%	23.1%	27.4%	31.7%
7500												%6.0	2.6%	4.6%	7.7%	9.4%	11.6%	14.2%	18.2%	21.6%	24.8%	28.0%	29.2%	37.0%
8000				·							%8.0	2.0%	4.2%	6.5%	10.3%	12.3%	14.7%	17.6%	22.1%	25.8%	29.0%	32.4%	37.0%	41.5%
0006										2.6%	4.4%	%9.9	9.8%	12.9%	17.6%	19.8%	22.6%	25.8%	30.9%	34.9%	38.0%	41.5%	45.9%	50.2%
10000									2.2%	%9.9	9.1%	11.8%	15.6%	19.7%	24.9%	27.2%	30.2%	33.2%	38.8%	42.8%	45.7%	48.7%	52.8%	56.3%
11000								%6'0	5.5%	11.5%	14.1%	17.3%	21.1%	25.8%	31.0%	33.1%	36.0%	38.8%	44.2%	48.0%	50.7%	53.6%	57.3%	60.4%

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Dam Boards Out (continued)	31000	2.2%
Jam	29000	
	27000	
wateren - ACLD	25000	
STEWS	23000	
	21000 23000 25000	
Callino	19000	(cfs)
CHINOOK SAIMON KEUUS DEWAL	17000	wning Flow (c
	15000 17000 17000	Spaw
oi rall-	14000	
entage	12000 13000	
Ferc	12000	

		Perce	intage (Percentage of Fall-run Chinook Salmon Redds Dewatered	run Ch	inook	Salmor	1 Redd	s Dewa	tered -	ACID	Dam E	- ACID Dam Boards Out (con
		12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	31000
	29000				Spawr	Spawning Flow (cfs)	(cfs)						2.2%
	27000											1.8%	5.3%
	25000										2.3%	6.4%	10.7%
	23000									%2.9	17.8%	31.2%	38.9%
	21000								4.2%	11.7%	23.9%	38.4%	46.3%
	19000							4.6%	10.8%	18.8%	30.4%	44.2%	51.1%
	17000						4.1%	11.3%	18.5%	26.1%	37.8%	51.5%	27.9%
	15000					5.3%	11.1%	18.5%	26.2%	33.5%	44.6%	22.6%	63.1%
	14000				2.6%	9.5%	14.8%	22.1%	28.9%	36.2%	46.8%	29.5%	64.7%
	13000			1.6%	4.8%	12.2%	16.9%	24.4%	31.3%	38.1%	48.4%	8.09	%6:39
	12000		1.6%	4.7%	9.0%	16.8%	20.6%	27.0%	32.9%	39.8%	20.0%	62.3%	67.2%
	11000	2.7%	5.3%	%0.6	13.6%	21.4%	24.8%	30.2%	35.3%	41.8%	51.6%	63.7%	68.4%
	10000	4.5%	7.7%	12.0%	16.4%	23.5%	26.9%	33.0%	38.5%	44.5%	54.1%	%6:39	%5.07
	0006	10.6%	14.4%	18.4%	22.5%	29.2%	31.9%	37.4%	41.8%	47.7%	22.0%	68.2%	72.6%
	8000	17.2%	20.9%	24.9%	28.9%	34.9%	36.6%	41.3%	45.4%	50.5%	59.3%	70.2%	74.7%
۰	7500	20.0%	23.2%	26.9%	30.7%	36.4%	38.2%	42.8%	46.8%	51.9%	%9.09	%6.07	75.3%
	7000	22.8%	25.8%	29.3%	32.9%	38.3%	40.0%	44.4%	48.3%	52.9%	61.3%	71.8%	76.1%
	6500	26.5%	29.2%	32.7%	36.1%	41.0%	42.4%	46.5%	50.4%	54.8%	63.0%	73.3%	77.77
	0009	30.9%	33.8%	37.3%	40.6%	45.0%	45.8%	49.5%	53.2%	57.2%	65.0%	75.4%	80.0%
	2500	35.8%	38.4%	41.7%	44.8%	48.3%	48.8%	52.3%	56.1%	60.1%	67.5%	77.3%	82.0%
	5250	37.7%	40.2%	43.5%	46.5%	20.0%	50.2%	53.5%	57.4%	%2.09	68.0%	78.2%	83.0%
	2000	40.6%	43.0%	46.1%	49.1%	52.2%	52.2%	55.2%	59.1%	63.3%	%9.02	79.4%	84.1%
	4750	43.3%	45.6%	48.6%	51.4%	54.0%	53.7%	26.6%	60.4%	64.5%	71.7%	80.3%	85.0%
	4500	48.3%	50.2%	52.8%	55.1%	57.1%	56.4%	29.0%	62.7%	66.2%	73.3%	81.8%	86.5%
	4250	51.8%	53.6%	26.0%	58.1%	29.6%	58.8%	61.3%	65.0%	68.5%	75.7%	83.1%	87.8%
	4000	54.3%	55.9%	58.2%	29.9%	61.2%	60.2%	62.7%	66.5%	70.4%	77.1%	84.1%	88.8%
	3750	26.9%	58.3%	%6.09	61.8%	62.7%	61.7%	64.0%	%2'.29	71.4%	.77.7%	84.9%	%9.68
	3500	60.1%	61.1%	63.0%	64.2%	64.9%	63.8%	%0.99	69.5%	73.0%	79.1%	86.2%	91.0%
	3250	62.9%	63.7%	65.3%	66.4%	89.99	65.7%	67.8%	71.3%	74.5%	80.4%	87.3%	92.0%

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Percentage of Fall-run Chinook Salmon Redds Dewatered . ACID Dam Roards In

													9		. , •	-,													
3250	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	10000	11000	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	
1.0%																													3500
2.0%	1.0%																												Pe 3750
3.3%	2.0%	0.9%																											rcenta 4000
4.7%	3.1%	1.6%	0.9%				,																						ge of H
6.2%	4.4%	2.5%	1.7%	0.8%																									all-run 4500
7.8%	5.7%	3.6%	2.6%	1.5%	0.8%																								4750
9.7%	7.4%	5.1%	3.8%	2.5%	1.6%	0.8%					.*																	Spav	Percentage of Fall-run Chinook Salmon Redds Dewatered - ACLD D 4000 4250 4500 4750 5000 5250 5500 6000 6500 7000
11.7%	9.2%	6.7%	5.3%	3.7%	2.6%	1.6%	0.7%																					Spawning Flow (cfs)	mon R 5250
13.6%	10.9%	8.3%	6.6%	5.0%	3.7%	2.4%	1.3%	0.6%																	,			(cfs)	edds D 5500
17.8%	14.8%	11.9%	10.0%	7.8%	6.0%	4.3%	2.9%	1.9%	1.2%																				ewater 6000
22.2%	18.8%	15.6%	13.5%	11.1%	8.9%	6.6%	4.9%	3.5%	2.7%	1.0%									.•										ed - A(6500
26.3%	22.8%	19.3%	16.9%	14.4%	11.9%	9.1%	7.2%	5.6%	4.4%	2.3%	1.1%														٠				7000
30.2%	23.9%	23.0%	20.4%	17.8%	15.0%	11.8%	9.6%	7.7%	6.2%	3.7%	2.1%	0.7%																	7500 8000
33.4%	29.8%	26.2%	23.5%	20.9%	17.8%	14.3%	11.9%	9.7%	8.1%	5.1%	3.3%	1.6%	0.6%																8000
39.5%	36.2%	32.8%	30.1%	27.5%	24.4%	20.3%	17.7%	15.3%	13.5%	9.8%	7.4%	5.0%	3.4%	2.0%															9000
43.5%	40.8%	37.7%	35.4%	33.1%	29.9%	25.7%	23.1%	20.4%	18.5%	14.5%	11.5%	8.6%	6.7%	4.9%	1.6%														10000
46.0%	43.6%	40.9%	38.7%	36.6%	33.6%	29.5%	26.9%	24.1%	22.3%	18.3%	15.0%	12.1%	9.9%	8.1%	3.8%	1.2%													11000
																												,	

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	Perc	Percentage of Fall-run Chinook Salmon Redds Dewatered	of Fal	I-run C	Thinook	Salmo	on Red	ds Dew	/atered	- ACI	D Dam	- ACID Dam Boards In (co	ತ
	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	31000	,
29000				Spaw	Spawning Flow (cfs)	(cfs)						1.5%	
27000											1.2%	3.3%	
25000									,	1.2%	3.0%	2.0%	
23000	-								0.7%	1.6%	3.6%	5.7%	
21000						•		1.9%	2.0%	2.6%	4.7%	%9.9	
19000							2.3%	4.8%	4.6%	2.0%	%6.9	8.4%	
17000						1.8%	2.0%	7.5%	6.5%	6.8%	8.5%	10.0%	
15000					2.8%	4.8%	7.7%	10.2%	8.6%	8.7%	10.4%	11.5%	
14000				1.8%	5.4%	7.0%	9.8%	11.8%	10.0%	86.6	11.4%	12.4%	
13000			1.0%	3.0%	%6.9	8.1%	11.1%	13.1%	11.0%	10.7%	12.1%	13.1%	
12000		1.0%	2.8%	5.4%	9.4%	10.0%	12.5%	14.0%	11.9%	11.5%	12.9%	13.9%	
11000	1.9%	3.4%	5.4%	8.2%	12.1%	12.2%	14.5%	15.6%	13.3%	12.8%	14.1%	15.0%	
10000	3.0%	4.9%	7.2%	9.8%	13.3%	13.8%	16.2%	17.4%	14.9%	14.5%	15.9%	16.7%	
0006	7.2%	9.2%	11.3%	13.6%	16.8%	16.8%	18.9%	19.6%	17.2%	16.8%	17.9%	18.5%	
8000	11.8%	13.7%	15.7%	17.9%	20.7%	20.2%	21.9%	22.4%	19.8%	19.4%	20.5%	21.4%	
7500	13.7%	15.3%	17.1%	19.3%	21.9%	21.5%	23.3%	23.9%	21.3%	21.0%	21.9%	22.7%	
7000	15.6%	17.0%	18.7%	20.7%	23.2%	22.8%	24.5%	25.1%	22.4%	22.1%	23.2%	24.0%	
6500	18.3%	19.5%	21.1%	23.0%	25.2%	24.7%	26.4%	27.1%	24.4%	24.2%	25.3%	26.3%	
0009	21.5%	22.7%	24.4%	26.2%	28.2%	27.5%	29.0%	29.8%	27.1%	27.1%	28.7%	29.8%	
5500	25.3%	26.4%	28.0%	29.7%	31.5%	31.0%	32.7%	33.8%	31.7%	31.9%	33.6%	35.1%	
5250	27.1%	28.2%	29.9%	31.8%	33.9%	33.5%	35.4%	36.8%	34.6%	35.0%	37.4%	39.0%	
2000	30.0%	31.2%	33.2%	35.3%	37.6%	37.6%	39.8%	41.7%	40.5%	41.3%	43.2%	45.1%	
4750	32.6%	34.0%	36.1%	38.3%	40.8%	41.1%	43.6%	45.7%	44.9%	46.0%	48.3%	20.3%	
4500	36.4%	37.6%	39.4%	41.4%	43.6%	43.9%	46.4%	48.7%	47.8%	49.1%	51.6%	53.7%	
4250	39.2%	40.3%	42.1%	43.9%	46.0%	46.4%	49.0%	51.3%	50.8%	52.5%	54.4%	26.5%	
4000	41.2%	42.2%	43.8%	45.5%	47.5%	47.9%	50.5%	53.1%	52.9%	54.5%	56.3%	28.5%	
3750	43.1%	43.9%	45.5%	47.0%	48.7%	49.1%	51.8%	54.3%	54.1%	25.6%	22.6%	29.8%	
3500	45.5%	46.0%	47.4%	48.8%	50.4%	20.8%	53.4%	55.9%	55.7%	57.2%	59.3%	61.6%	
3250	47.6%	48.0%	49.3%	20.5%	52.0%	52.5%	55.1%	27.6%	57.4%	59.0%	61.1%	63.3%	

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Percentage of Late-Fall-run Chinook Salmon Redds Dewatered - ACID Dam Boards Out

3250	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	10000	11000	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	
0.9%																													3500
1.6%	0.9%	2																											3750
2.6%	1.6%	0.8%																											4000
3.6%	2.4%	1.1%	0.7%																										4250
4.9%	3.4%	2.0%	1.2%	0.6%																									4500
6.3%	4.5%	2.9%	2.0%	1.1%	0.6%																								4750
8.0%	6.0%	4.1%	3.0%	1.9%	1.2%	0.6%																						Spaw	5000
9.8%	7.6%	5.5%	4.2%	3.0%	2.1%	1.3%	0.5%										,											Spawning Flow (cfs)	5250
77./%	9.3%	7.0%	5.5%	4.1%	3.1%	2.0%	1.0%	0.5%																				(cfs)	5500
%6.GL	13.1%	10.5%	8.8%	6.9%	5.5%	4.0%	2.6%	1.8%	1.3%																				6000
20.1%	17.1%	14.2%	12.1%	10.0%	8.3%	6.3%	4.6%	3.5%	2.7%	0.9%				,													•		6500
24.1%	21.0%	17.8%	15.6%	13.4%	11.3%	9.0%	7.0%	5.6%	4.6%	2.3%	1.0%																		7000
20.0%	24.9%	21.6%	19.2%	16.9%	14.6%	11.8%	9.6%	7.9%	6.7%	3.8%	2.1%	0.8%																	7500
31.5%	28.2%	25.0%	22.5%	20.1%	17.7%	14.7%	12.2%	10.4%	8.9%	5.5%	3.5%	1.8%	0.7%																8000
37.0%	35.0%	32.0%	29.5%	27.3%	24.8%	21.5%	18.9%	16.9%	15.3%	11.2%	8.3%	5.9%	3.9%	2.2%															9000
	40.2%	37.5%	35.3%	33.3%	30.8%	27.6%	25.2%	23.1%	21.5%	17.1%	13.4%	10.4%	7.9%	5.5%	1.7%														10000
43.0%	45.5%	40.7%	38.7%	36.8%	34.5%	31.5%	29.3%	27.4%	25.8%	21.7%	17.6%	14.4%	11.5%	8.9%	3.9%	1.2%													11000

Dewatering flow (cfs)

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Cate-Fall-run Chinook Salmon Redds Dewatered - ACID Dam Boards Out (continued)	•		-																	•		
oards (31000	1.6%	4.4%	9.4%	32.6%	39.1%	43.2%	49.1%	54.1%	25.9%	57.2%	28.9%	%2.09	62.9%	65.9%	%0.69	70.3%	71.6%	74.0%	77.3%	80.0%	81.1%
Dam B	29000		1.4%	5.2%	25.7%	32.1%	36.9%	42.9%	48.2%	50.1%	51.5%	53.3%	55.1%	57.4%	60.5%	63.7%	65.2%	%9.99	69.1%	72.1%	74.9%	76.0%
ACID	27000			1.8%	14.5%	20.0%	25.2%	31.1%	36.7%	38.8%	40.3%	42.2%	44.2%	46.7%	20.0%	53.7%	55.4%	27.0%	29.7%	62.7%	65.9%	%0:29
tered -	25000				5.1%	9.3%	14.4%	20.3%	26.0%	28.3%	29.9%	31.8%	34.0%	36.7%	40.2%	44.3%	46.3%	48.0%	20.9%	54.1%	57.7%	58.9%
s Dewa	23000		-			3.1%	7.1%	13.1%	18.8%	21.1%	22.7%	24.5%	26.9%	29.8%	33.2%	37.6%	39.6%	41.6%	44.5%	47.7%	51.2%	52.6%
ı Redd	21000	,					2.5%	7.0%	11.7%	14.6%	16.2%	18.7%	21.8%	24.0%	28.1%	32.5%	34.5%	36.4%	39.2%	42.3%	45.7%	47.0%
Salmo	19000	v (cfs)				ł	٠	3.5%	6.7%	9.8%	11.3%	14.2%	17.4%	19.6%	23.7%	28.3%	30.4%	32.4%	35.1%	38.3%	41.6%	42.9%
inook	17000	Spawning Flow (cfs)							3.3%	6.4%	8.3%	11.7%	15.2%	17.0%	21.5%	26.3%	28.3%	30.3%	33.1%	36.4%	39.7%	41.1%
run Ch	15000	Spav								1.9%	3.4%	6.5%	10.0%	12.1%	16.7%	21.9%	24.0%	26.1%	29.1%	32.6%	36.1%	37.5%
e-Fall-	14000										1.1%	3.4%	6.7%	8.8%	13.6%	18.9%	21.1%	23.3%	26.4%	29.9%	33.2%	34.9%
of Lat	13000											1.2%	4.1%	2.6%	10.5%	16.0%	18.4%	20.7%	23.8%	27.5%	31.2%	32.5%
Percentage of I	12000												2.3%	3.1%	7.8%	13.3%	16.0%	18.6%	21.7%	25.5%	29.4%	30.8%
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USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006
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Percentage of Late-Fall-run Chinook Salmon Redds Dewatered - ACID Dam Boards In

9	3050	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	, 10000	11000	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	
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2	S 80%	1.6%	0.8%																		•									4000
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5	4 0%	3.4%	2.0%	1.3%	0.7%																									4500
; }	6 Sec. 1	4.5%	2.9%	2.0%	1.2%	0.6%																								4750
	7 80/	5.9%	4.1%	3.0%	2.0%	1.3%	0.6%														•								Spaw	5000
	0 79 7	7.4%	5.5%	4.2%	3.0%	2.1%	1.3%	0.6%																		٠			Spawning Flow (cfs)	5250
	11 20/	9.0%	6.9%	5.4%	4.1%	3.1%	2.1%	1.1%	0.5%																				(cfs)	5500
2.1 %	707 215	12.5%	10.1%	8.4%	6.7%	5.3%	3.9%	2.6%	1.7%	1.2%																				6000
10.976	18 00/	16.1%	13.4%	11.5%	9.6%	7.9%	6.0%	4.4%	3.3%	2.5%	0.9%																			6500
22.070	э л %	19.6%	16.7%	14.7%	12.6%	10.7%	8.5%	6.6%	5.2%	4.3%	2.1%	0.9%																		7000
20.076	26.00/	23.1%	20.1%	17.9%	15.8%	13.6%	11.1%	8.9%	7.3%	6.1%	3.4%	1.9%	0.8%															,		7500
23.170	20 40	26.1%	23.1%	20.9%	18.7%	16.4%	13.7%	11.3%	9.5%	8.1%	5.0%	3.1%	1.6%	0.7%																8000
0,6.40	34 00/	32.3%	29.5%	27.3%	25.2%	22.9%	19.9%	17.4%	15.3%	13.7%	10.0%	7.4%	5.2%	3.5%	2.0%		•													9000
JB.470	30 70/	37.1%	34.6%	32.7%	30.8%	28.4%	25.4%	22.9%	20.7%	19.1%	15.1%	11.8%	9.1%	6.9%	4.9%	1.5%				. •										10000
42.370	3 9	40.1%	37.8%	36.0%	34.2%	32.0%	29.1%	26.7%	24.4%	22.8%	19.0%	15.4%	12.5%	9.9%	7.7%	3.3%	1.0%							•						11000

Dewatering flow (cfs)

USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006

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			opaw		(cls)					1.3%	4.0%
									1.3%	3.4%	6.4%
								1.1%	2.1%	4.3%	7.4%
							2.0%	2.7%	3.5%	5.8%	8.4%
						1.7%	4.2%	5.1%	5.8%	8.1%	10.5%
					2.4%	4.3%	7.5%	7.7%	8.2%	10.6%	13.2%
				2.5%	4.2%	6.9%	10.6%	10.3%	10.8%	13.3%	16.0%
-			.7%	5.1%	6.7%	9.2%	12.4%	12.1%	12.4%	15.0%	17.7%
0.9%		Ċ.	2.8%	6.5%	7.8%	10.4%	13.7%	13.3%	13.6%	16.3%	19.0%
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2.7% 4.6% 7.0% 9.8		9.6	9.8%	13.3%	14.3%	16.7%	19.3%	19.0%	19.4%	22.3%	25.1%
6.6% 8.7% 11.1% 13.	•	13.	13.6%	17.0%	17.7%	20.1%	22.2%	22.1%	22.8%	25.8%	28.7%
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16.0% 17.6% 19.6% 21.8%		21.8	%	24.9%	25.5%	27.8%	30.2%	30.2%	31.1%	34.3%	37.1%
18.8% 20.3% 22.3% 24.5%		24.5	%2	27.4%	28.1%	30.5%	33.0%	33.3%	34.5%	37.8%	40.8%
22.2% 23.7% 25.6% 27.		27.	27.7%	30.6%	31.3%	33.7%	36.4%	37.0%	38.6%	42.4%	45.9%
25.9% 27.3% 28.9% 31.		3	31.4%	34.2%	35.1%	37.8%	41.0%	42.1%	43.9%	48.0%	51.9%
27.5% 28.9% 30.9% 33.		33.	33.2%	36.3%	37.3%	40.2%	43.6%	44.8%	46.9%	51.4%	55.5%
29.8% 31.4% 33.5% 35.		35.	35.9%	39.1%	40.5%	43.6%	47.5%	49.3%	51.9%	56.3%	%9.09
32.2% 33.8% 36.0% 38		38	38.5%	41.9%	43.5%	46.8%	20.6%	53.2%	25.9%	%9.09	65.1%
34.9% 36.5% 38.6% 41.	•	41.	41.1%	44.4%	46.1%	49.6%	53.9%	56.3%	59.2%	64.1%	68.7%
37.1% 38.7% 40.8% 43.2%	·	43.2	5%	46.5%	48.3%	51.9%	56.4%	29.0%	62.2%	%6.99	71.5%
38.8% 40.4% 42.4% 44.8%		44.8	%	48.1%	20.0%	53.6%	58.3%	61.1%	64.3%	68.9%	73.5%
40.5% 42.0% 44.0% 46.		46.	46.3%	49.6%	51.5%	55.1%	29.7%	62.6%	65.6%	70.4%	75.1%
42.6% 44.0% 46.0% 48		48	48.2%	51.5%	53.5%	57.2%	61.8%	64.6%	67.8%	72.6%	77.3%
44.6% 46.0% 47.9% 50		20	50.1%	53.4%	55.4%	59.2%	63.7%	66.8%	%2.69	74.4%	79.1%

USFWS, SFWO, Energy Planning and Instream Flow Branch Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

15000 17000 21000 23000 25000 27000 29000 11000 14000 19000 8000 9000 10000 12000 13000 3500 3750 Percentage of Winter-run Chinook Salmon Redds Dewatered - ACID Dam Boards Out 4000 4250 4500 4750 5000 Spawning Flow (cfs) 5250 5500 6000 6500 7000

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Dewatering flow (cfs)

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June 22, 2006	Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juven	USFWS, SFWO, Energy Planning and Instream Flow Branch
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USFWS, SFWO, Energy Planning and Instream Flow Branch Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

3250	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	10000	11000	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	•
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8.8%	5.3%	2.8%	1.9%	1.1%	0.8%	0.4%	0.3%																					Spawning Flow (cfs)	almon 5250
10.2%	6.3%	3.5%	2.5%	1.5%	1.1%	0.7%	0.5%	0.3%																				/ (cfs)	Redds 5500
13.0%	8.5%	5.1%	3.8%	2.6%	1.9%	1.3%	1.0%	0.7%	0.4%																				Percentage of Winter-run Chinook Salmon Redds Dewatered - 50 4000 4250 4500 4750 5000 5250 5500 6000 6500
16.0%	11.0%	7.0%	5.4%	3.9%	2.9%	2.1%	1.6%	1.2%	0.9%	0.3%																			
18.9%	13.6%	9.3%	7.5%	5.6%	4.3%	3.1%	2.5%	1.9%	1.5%	0.7%	0.2%																		ACID I 7000
21.9%	16.2%	11.7%	9.8%	7.6%	5.9%	4.5%	3.7%	2.9%	2.3%	1.3%	0.5%	0.3%																	Dam Boards In 7500 8000
24.7%	18.9%	14.2%	12.2%	9.7%	7.9%	6.1%	5.0%	3.9%	3.2%	1.9%	1.0%	0.5%	0.3%																ards L
30.5%	24.7%	19.9%	17.7%	15.0%	12.6%	10.5%	9.0%	7.3%	6.1%	4.0%	2.4%	1.4%	0.9%	0.4%															9000
35.9%	30.4%	25.9%	23.7%	20.7%	18.1%	15.7%	14.0%	11.9%	10.5%	8.0%	5.8%	3.8%	2.9%	2.1%	1.3%														10000
40.1%	34.8%	30.5%	28.3%	25.2%	22.4%	20.0%	18.1%	15.9%	14.3%	11.3%	8.8%	6.1%	4.8%	3.7%	2.4%	0.9%													11000

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	25000									1	0.3%	%6.0	1.6%	
	23000									0.4%	%9.0	1.1%	1.9%	
	21000								%6.0	1.6%	2.1%	3.0%	4.0%	
	19000							1.0%	2.5%	3.6%	4.4%	5.5%	%9.9	
	17000						%6.0	2.5%	4.9%	6.5%	7.7%	9.1%	10.4%	
	15000					0.7%	2.1%	3.9%	6.8%	8.6%	10.1%	11.6%	13.0%	
	14000				0.5%	1.7%	3.5%	5.5%	8.2%	10.1%	11.7%	13.4%	14.9%	
	13000			0.5%	1.1%	2.6%	4.4%	6.7%	%9.6	11.7%	13.5%	15.3%	17.0%	
	12000		0.5%	1.2%	2.2%	4.2%	6.4%	9.1%	12.1%	14.6%	16.8%	19.1%	21.1%	
	11000	1.1%	2.0%	3.1%	4.6%	7.6%	10.5%	13.8%	17.4%	20.6%	23.5%	26.7%	29.4%	
	10000	2.2%	3.6%	5.2%	7.0%	10.5%	14.0%	17.7%	18.6%	25.4%	28.9%	32.6%	35.8%	
	0006	4.0%	2.6%	7.4%	9.4%	13.3%	16.9%	20.8%	24.9%	28.7%	32.8%	36.3%	39.6%	
_	8000	2.7%	7.6%	9.7%	11.8%	15.9%	19.6%	23.5%	27.7%	31.6%	35.7%	39.1%	42.4%	
_	7500	7.0%	%0.6	11.2%	13.5%	17.7%	21.4%	25.2%.	29.3%	33.2%	37.2%	40.7%	44.0%	
	2000	8.7%	10.9%	13.3%	15.7%	20.1%	23.7%	27.5%	31.5%	35.4%	39.4%	42.9%	46.2%	
	6500	11.8%	14.3%	16.8%	19.3%	23.7%	27.2%	30.7%	34.7%	38.4%	42.3%	45.9%	49.3%	
	0009	14.5%	17.1%	19.8%	22.3%	26.8%	30.2%	33.7%	37.5%	41.3%	45.2%	48.8%	52.2%	
	5500	17.9%	20.7%	23.5%	26.1%	30.5%	33.9%	37.4%	41.2%	44.8%	48.7%	52.4%	25.8%	
	5250	19.5%	22.5%	25.2%	27.9%	32.2%	35.6%	39.0%	42.8%	46.4%	50.3%	54.1%	57.5%	
	2000	21.7%	24.6%	27.4%	29.9%	34.2%	37.4%	40.8%	44.6%	48.0%	51.9%	55.7%	59.1%	
	4750	23.7%	26.7%	29.5%	32.1%	36.3%	39.5%	42.8%	46.6%	49.9%	53.9%	27.6%	61.1%	
	4500	26.3%	29.3%	32.0%	34.6%	38.6%	41.7%	45.0%	48.7%	52.0%	56.0%	59.8%	63.2%	
	4250	29.2%	32.2%	34.9%	37.4%	41.4%	44.4%	47.5%	51.2%	54.4%	58.5%	62.3%	65.7%	
	4000	32.2%	35.3%	38.0%	40.4%	44.2%	47.2%	50.3%	53.9%	57.0%	61.1%	65.0%	68.5%	
	3750	34.4%	37.3%	40.0%	42.4%	46.1%	49.0%	52.1%	55.7%	58.8%	62.8%	%2'99	70.2%	
	3500	38.5%	41.1%	43.9%	46.1%	49.6%	52.3%	55.3%	58.8%	61.9%	65.9%	%6.69	73.5%	
	3250	43.4%	46.0%	48.4%	50.3%	53.5%	26.0%	58.9%	62.4%	65.4%	69.5%	73.7%	77.2%	

Dewatering flow (cfs)

USFWS, SFWO, Energy Planning and Instream Flow Branch Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

June 22, 2006	Sacrament	USFWS, S
006	Sacramento River (Keswick Dam to Battle Creel	USFWS, SFWO, Energy Planning and Instream

										i	Dew	/ater	ing	flow	(cfs														
3250	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	10000	11000	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	
1.2%															ļ														3500
2.6%	1.6%																		r										3750
3.7%	2.4%	0.5%																											4000
4.9%	3.2%	1.2%	0.8%																										4250
6.8%	4.7%	2.5%	1.9%	1.1%																					÷				4500
8.9%	6.4%	3.8%	2.9%	2.2%	1.1%																								.4750
10.9%	8.0%	5.3%	4.0%	3.2%	1.9%	0.8%				-																		Spa	5000
13.3%	10.2%	7.3%	5.7%	4.8%	3.3%	2.0%	1.1%									-								,				Spawning Flow (cfs)	5250
15.7%	12.4%	9.1%	7.3%	6.2%	4.5%	2.8%	1.8%	0.8%					•															w (cfs)	4250 4500 .4750 5000 5250 5500 6000 6500 7000 7500
19.9%	16.5%	12.7%	10.5%	9.3%	7.1%	5.1%	3.7%	2.4%	1.5%																				6000
23.4%	19.9%	15.9%	13.4%	12.0%	9.6%	7.4%	5.8%	4.2%	3.2%	1.3%																			6500
26.2%	22.8%	18.9%	16.0%	14.6%	12.0%	9.7%	8.0%	6.2%	5.0%	2.7%	2.8%																		7000
28.5%	25.1%	21.2%	18.2%	16.7%	14.0%	11.6%	9.7%	7.7%	6.1%	3.8%	1.3%	0.5%																	7500
31.1%	27.7%	23.9%	20.8%	19.1%	16.3%	13.8%	11.8%	9.4%	7.8%	5.3%	2.6%	1.3%	0.7%																8000
37.2%	33.8%	30.3%	27.1%	25.3%	22.4%	19.8%	17.7%	14.9%	13.0%	10.2%	6.9%	4.8%	3.8%	2.8%															9000
43.5%	40.1%	36.5%	33.5%	31.5%	28.5%	25.8%	23.8%	21.1%	19.1%	15.9%	12.1%	9.4%	8.1%	6.7%	2.6%														10000
49.8%	46.2%	42.4%	39.5%	37.3%	34.2%	31.4%	26.6%	26.8%	24.6%	21.2%	17.2%	14.3%	12.7%	10.9%	5.3%	1.9%													11000

	-	Percent	age of	Steelhe	ad Rec	lds Dev	vatere	1 - AC	D Dan	Board	ls Out	Percentage of Steelhead Redds Dewatered - ACID Dam Boards Out (continued)	(pai
	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	31000	•
29000				Spawi	Spawning Flow (cfs)	(cfs)						5.1%	
27000											3.9%	9.5%	
25000										4.0%	9.3%	14.6%	
23000									13.5%	20.5%	26.9%	31.2%	
21000								8.9%	20.0%	29.2%	36.3%	39.9%	
19000							7.3%	13.3%	26.4%	35.7%	43.1%	46.6%	
17000						4.8%	13.4%	20.8%	34.1%	42.5%	49.7%	53.0%	
15000					5.0%	10.9%	20.7%	29.5%	42.1%	49.3%	55.8%	58.8%	
14000				3.0%	9.5%	15.6%	25.1%	32.3%	44.5%	51.7%	58.3%	61.5%	
13000			5.4%	5.4%	12.7%	19.7%	29.1%	36.4%	48.0%	54.6%	61.2%	64.1%	
12000		3.1%	6.3%	10.4%	18.9%	25.1%	33.9%	38.9%	50.3%	26.5%	63.0%	65.7%	
11000	2.8%	6.8%	10.9%	15.7%	24.3%	29.5%	37.4%	42.0%	52.8%	58.7%	65.1%	%2'.29	
10000	4.6%	8.9%	13.4%	18.9%	27.7%	33.7%	41.4%	45.6%	55.7%	61.7%	68.3%	%8'02	
0006	9.6%	14.5%	19.7%	25.7%	35.2%	40.4%	47.2%	50.7%	60.2%	65.3%	71.1%	73.5%	
8000	16.3%	22.0%	27.7%	33.4%	42.1%	46.4%	52.7%	. %6'99	64.6%	69.5%	75.0%	77.2%	
7500	18.2%	24.1%	30.0%	35.8%	44.4%	48.2%	54.1%	57.3%	66.2%	71.1%	%0'92	78.2%	
2000	19.9%	25.7%	31.6%	37.5%	46.2%	50.2%	26.0%	59.1%	67.5%	72.2%	77.3%	79.4%	
6500	22.9%	28.7%	34.5%	40.4%	48.6%	52.6%	58.2%	61.0%	69.2%	74.0%	79.2%	81.4%	
0009	26.6%	32.3%	38.4%	44.7%	53.8%	58.8%	64.6%	%1.79	74.9%	79.2%	84.3%	86.8%	
2500	30.1%	36.0%	42.2%	48.8%	58.2%	63.6%	69.2%	71.9%	78.2%	82.1%	87.2%	89.9%	
5250	32.6%	38.7%	45.2%	51.9%	61.3%	66.1%	70.8%	73.2%	79.3%	82.9%	88.1%	%8.06	
5000	35.4%	41.7%	48.2%	25.0%	64.1%	68.2%	72.8%	75.2%	82.1%	85.0%	88.8%	91.6%	
4750	37.3%	43.7%	50.2%	22.0%	66.1%	70.1%	74.6%	77.0%	83.1%	85.5%	89.2%	91.9%	
4500	40.3%	46.9%	53.7%	60.5%	69.4%	73.1%	77.4%	79.4%	84.3%	86.3%	89.7%	92.2%	
4250	43.6%	50.3%	57.3%	64.1%	72.3%	75.6%	79.8%	82.0%	86.8%	88.3%	91.0%	93.3%	
4000	46.0%	52.9%	%0.09	89.99	74.9%	78.2%	82.1%	84.1%	88.1%	89.4%	91.6%	93.7%	
3750	49.0%	25.9%	63.0%	69.7%	77.8%	80.9%	84.3%	85.9%	88.9%	89.7%	91.7%	93.8%	
3500	52.9%	%0.09	67.1%	73.6%	81.4%	84.0%	86.4%	87.4%	89.9%	90.5%	92.3%	94.0%	
3250	26.6%	63.7%	70.7%	76.8%	84.2%	86.5%	88.5%	89.1%	91.0%	91.3%	93.1%	94.7%	

Dewatering flow (cfs)

USFWS, SFWO, Energy Planning and Instream Flow Branch Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

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3250	3500	3750	4000	4250	4500	4750	5000	5250	5500	6000	6500	7000	7500	8000	9000	10000	11000	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	
1.1%																													3500
2.3%	1.4%																												3750
3.3%	2.2%	0.6%	J																										J 4000
4.7%	3.2%	1.3%	0.9%																										Percent 4250
6.5%	4.6%	2.6%	2.1%	1.3%																									tage of
8.7%	6.4%	4.1%	3.3%	2.6%	1.4%																								Steelh
11.0%	8.4%	5.9%	4.7%	4.0%	2.7%	1.5%								•														Spav	ead Re
13.6%	10.8%	8.1%	6.7%	5.8%	4.2%	2.9%	1.7%																					Spawning Flow (cfs)	Percentage of Steelhead Redds Dewatered - ACID Dam Boards In 4250 4500 4750 5000 5250 5500 6000 6500 7000 7500
16.0%	13.0%	10.0%	8.3%	7.2%	5.5%	3.8%	2.4%	1.1%																				(cfs)	watere
20.3%	17.1%	13.6%	11.6%	10.3%	8.2%	6.2%	4.4%	2.6%	1.5%																				d - AC 6000
23.9%	20.6%	17.0%	14.6%	13.2%	10.8%	8.5%	6.5%	4.6%	3.2%	1.3%																			(D Dan 6500
26.9%	23.7%	20.0%	17.4%	15.9%	13.3%	11.0%	8.8%	6.5%	4.8%	2.7%	2.7%																		n Boar 7000
29.3%	26.1%	22.5%	19.7%	18.1%	15.4%	12.9%	10.6%	8.0%	6.2%	3.8%	1.4%	0.5%																	ds In 7500
31.8%	28.6%	25.1%	22.2%	20.5%	17.6%	15.1%	12.6%	9.6%	7.7%	5.1%	2.5%	1.3%	0.7%																8000
37.6%	34.5%	31.2%	28.3%	26.5%	23.6%	20.9%	18.3%	15.0%	12.8%	9.9%	6.9%	4.9%	4.0%	3.0%															9000
42.3%	39.2%	35.9%	33.3%	31.3%	28.4%	25.7%	23.1%	19.7%	17.5%	14.3%	10.8%	8.4%	7.3%	5.9%	2.2%														10000
46.7%	43.5%	40.3%	37.8%	35.7%	32.7%	30.0%	27.5%	24.0%	21.6%	18.3%	14.8%	12.2%	10.8%	9.2%	4.4%	1.6%													11000

Dewatering flow (cfs)

3250 3500

50.5% 47.4%

53.5% 50.6%

55.6% 52.9%

54.1% 52.3%

49.5% 48.1%

46.8% 45.6%

39.1% 38.2%

38.3% 37.6%

37.7% 37.0%

39.2% 38.5%

41.3% 42.3%

54.1% 56.3%

Dewatering flow (cfs)

		Perce	ntage o	f Steell	nead Ro	edds D	ewater	ed - A(ID Da	m Boai	ds In (Percentage of Steelhead Redds Dewatered - ACID Dam Boards In (continued)
	12000	13000	14000	15000	17000	19000	21000	23000	25000	27000	29000	31000
29000				Spaw	Spawning Flow (cfs)	(cfs)						2.2%
27000											1.8%	3.8%
25000										1.7%	3.4%	5.4%
23000									0.9%	2.2%	3.8%	5.7%
21000								1.4%	1.8%	2.9%	4.4%	6.3%
19000				•			3.0%	3.7%	3.8%	5.1%	6.7%	8.4%
17000						1.9%	5.1%	5.8%	5.6%	6.8%	8.3%	10.0%
15000					2.6%	4.2%	7.2%	7.9%	7.4%	8.3%	9.4%	10.9%
14000				2.1%	5.1%	6.6%	9.1%	9.0%	8.3%	9.2%	10.3%	11.9%
13000			3.7%	3.6%	6.8%	8.3%	10.7%	10.5%	9.6%	10.3%	11.3%	12.7%
12000		2.2%	4.3%	6.7%	10.1%	10.9%	12.9%	11.4%	10.4%	10.9%	11.9%	13.2%
11000	2.3%	5.0%	7.5%	10.1%	13.1%	13.1%	14.5%	12.8%	11.5%	11.9%	12.8%	14.1%
10000	3.6%	6.6%	9.2%	12.1%	15.1%	15.3%	16.4%	14.5%	12.9%	13.4%	14.3%	15.5%
9000	7.6%	10.8%	13.6%	16.6%	19.4%	18.7%	19.3%	16.8%	15.2%	15.4%	15.9%	17.0%
8000	13.1%	16.6%	19.5%	21.9%	23.7%	22.2%	22.5%	19.7%	18.0%	18.1%	18.5%	19.5%
7500	14.8%	18.3%	21.2%	23.7%	25.2%	23.5%	23.5%	20.7%	19.1%	19.3%	19.4%	20.4%
7000	16.2%	19.6%	22.5%	24.9%	26.4%	24.7%	24.5%	21.7%	19.9%	20.2%	20.4%	21.4%
6500	18.7%	22.1%	24.8%	27.1%	28.1%	26.2%	25.9%	22.9%	21.2%	21.5%	21.7%	22.8%
6000	21.9%	25.1%	27.8%	30.2%	31.3%	29.7%	29.4%	26.3%	24.3%	24.5%	24.8%	26.0%
5500	25.3%	28.4%	31.1%	33.5%	34.5%	32.8%	32.3%	28.9%	26.8%	27.0%	27.3%	28.8%
5250	27.9%	31.1%	33.8%	36.2%	36.9%	34.8%	33.8%	30.3%	28.2%	28.4%	28.9%	30.4%
5000	31.2%	34.4%	37.2%	39.4%	39.8%	37.2%	36.2%	32.8%	31.1%	31.1%	31.1%	32.8%
4750	33.7%	37.0%	39.7%	41.8%	42.1%	39.4%	38.2%	34.8%	32.9%	32.8%	33.0%	34.8%
4500	36.6%	39.8%	42.6%	44.6%	44.5%	41.5%	40.1%	36.5%	34.2%	34.0%	34.0%	35.8%
4250	39.5%	42.8%	45.5%	47.3%	46.6%	43.2%	41.7%	38.2%	36.0%	35.6%	35.4%	37.1%
4000	41.7%	45.1%	47.7%	49.4%	48.3%	44.8%	43.2%	39.4%	37.0%	36.5%	36.2%	37.8%
3750	44.2%	47.4%	49.9%	51.4%	50.6%	46.3%	44.4%	40.4%	37.6%	37.0%	36.5%	38.1%
1	i))		1						. -	

APPENDIX F REVISED SITE SPAWNING HABITAT MODELING RESULTS

Salt Creek Study Site Boards Out WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	4,612	2,586	1,411	1,228
3,500	4,481	2,439	1,521	1,216
3,750	4,354	2,303	1,581	1,214
4,000	4,196	2,160	1,612	1,200
4,250	4,027	2,031	1,628	1,191
4,500	3,854	1,919	1,627	1,193
4,750	3,699	1,832	1,613	1,196
5,000	3,564	1,764	1,595	1,199
5,250	3,426	1,701	1,567	1,192
5,500	3,286	1,642	1,524	1,185
6,000	3,376	1,049	353	1,222
6,500	2,750	1,428	1,363	1,143
7,000	2,472	1,291	1,230	1,101
7,500	2,251	1,191	1,111	1,007
8,000	2,082	1,134	1,010	910
9,000	1,738	996	863	660
10,000	1,464	868	731	573
11,000	1,255	767	636	532
12,000	1,099	682	557	486
13,000	1,006	637	511	461
14,000	960	623	483	451
15,000	868	594	462	434
17,000	705	522	441	·383
19,000	809	640	537	383
21,000	693	588	496	330
23,000	891	802	616	404
25,000	780	733	698	400
27,000	978	937	963	398
29,000	913	878	854	360
31,000	858	830	857	321

Salt Creek Study Site Boards In WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	4,601	2,489	1,239	1,282
3,500	4,519	2,367	1,338	1,240
3,750	4,412	2,235	1,408	1,234
4,000	4,397	2,178	1,407	1,209
4,250	4,219	2,035	1,454	1,227
4,500	4,110	1,955	1,459	1,241
4,750	3,976	1,876	1,463	1,238
5,000	3,819	1,795	1,449	1,227
5,250	3,648	1,709	1,417	1,221
5,500	3,478	1,623	1,371	1,207
6,000	3,640	1,270	509	1,210
6,500	3,433	1,099	418	1,195
7,000	3,226	960	330	1,190
7,500	2,939	767	250	1,162
8,000	2,214	1,115	955	1,040
9,000	1,832	936	771	789
10,000	1,547	816	640	603
11,000	1,348	734	545	573
12,000	1,225	695	492	558
13,000	1,142	704	499	546
14,000	1,007	643	453	490
15,000	1,422	848	522	801
17,000	1,156	808	569	598
19,000	1,048	823	615	520
21,000	981	834	622	486
23,000	1,134	1,030	936	502
25,000	1,046	977	879	446
27,000	1,083	1,028	1,048	534
29,000	892	853	920	377
31,000	873	842	893	444

Upper Lake Redding Study Site Boards Out WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	205,573	187,719	144,932	48,196
3,500	199,927	182,105	152,226	46,763
3,750	193,214	175,813	158,530	45,653
4,000	185,327	168,820	164,003	44,769
4,250	177,601	162,053	168,119	44,112
4,500	169,639	155,168	171,384	43,606
4,750	161,428	148,229	173,808	43,207
5,000	150,847	139,932	176,103	42,938
5,250	143,725	133,845	176,545	42,862
5,500	135,913	127,606	176,685	42,851
6,000	119,816	115,290	175,220	42,841
6,500	105,647	104,548	171,934	42,754
7,000	92,469	94,883	167,505	42,399
7,500	80,488	86,220	162,376	41,731
8,000	70,069	78,667	156,030	40,707
9,000	53,400	66,211	142,066	37,820
10,000	41,117	56,837	127,929	33,768
11,000	32,055	49,284	113,868	28,661
12,000	25,698	43,433	100,691	23,241
13,000	20,979	38,886	88,343	17,315
14,000	17,541	35,126	77,460	11,723
15,000	14,686	31,743	67,418	7,114
17,000	11,292	26,571	50,825	1,444
19,000	9,734	22,390	38,509	292
21,000	8,861	18,619	29,738	183
23,000	7,706	14,822	22,993	140
25,000	6,662	11,217	18,263	128
27,000	5,491	8,265	14,643	111
29,000	3,867	5,226	11,637	82
31,000	2,014	2,870	9,404	65

Upper Lake Redding Study Site Boards In WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	151,666	125,042	39,306	78,678
3,500	164,283	133,532	44,252	76,016
3,750	175,511	140,956	49,198	73,226
4,000	185,553	147,399	54,219	70,263
4,250	193,612	152,431	59,035	67,310
4,500	200,110	156,224	63,948	64,476
4,750	205,260	158,788	68,765	61,837
5,000	208,977	160,318	73,301	59,369
5,250	211,499	160,879	77,568	57,106
5,500	213,158	160,803	81,759	54,973
6,000	213,524	158,325	89,258	51,385
6,500	210,195	153,724	96,090	48,691
7,000	204,420	147,507	101,660	46,720
7,500	196,791	140,417	106,315	45,136
8,000	188,193	132,789	110,043	43,961
9,000	169,240	117,025	114,353	43,002
10,000	149,770	102,490	115,064	42,938
11,000	131,733	89,689	112,457	42,938
12,000	114,806	78,484	108,233	. 42,948
13,000	100,012	68,991	102,641	42,959
14,000	86,996	60,932	96,197	42,970
15,000	76,361	54,133	89,528	42,981
17,000	59,445	42,884	75,219	42,895
19,000	46,838	34,393	61,287	42,259
21,000	38,035	27,411	48,131	41,030
23,000	32,023	21,690	35,999	39,231
25,000	27,788	16,992	25,127	36,667
27,000	23,995	12,951	15,602	33,068
29,000	21,377	9,331	7,783	29,620
31,000	19,308	6,164	2,929	26,646

Lower Lake Redding Study Site Boards Out WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	19,287	30,008	22,972	5,732
3,500	18,910	29,598	23,037	5,657
3,750	18,640	29,167	23,069	5,463
4,000	18,490	28,726	23,047	5,269
4,250	18,403	28,273	23,144	4,967
4,500	18,188	27,627	23,144	4,698
4,750	17,940	26,905	23,241	4,579
5,000	17,660	26,064	23,392	4,472
5,250	17,347	25,288	23,608	4,310
5,500	17,164	24,696	23,931	4,202
6,000	16,561	23,327	24,340	4,051
6,500	15,149	21,377	23,952	3,771
7,000	13,565	19,384	23,306	3,491
7,500	12,111	17,574	22,325	3,362
8,000	11,033	16,076	21,313	3,308
9,000	9,719	13,921	19,265	3,125
10,000	8,835	12,510	17,337	3,049
11,000	8,781	12,003	15,699	3,028
12,000	8,200	11,109	14,460	2,855
13,000	8,275	10,850	13,275	2,888
14,000	8,361	10,656	12,316	2,823
15,000	8,361	10,408	11,583	2,715
17,000	9,072	10,408	10,753	2,942
19,000	10,861	11,982	10,658	3,834
21,000	12,553	13,900	10,947	4,881
23,000	14,955	16,776	12,391	6,196
25,000	26,969	29,803	16,098	11,454
27,000	41,451	45,879	25,159	12,833
29,000	48,519	54,176	37,981	12,176
31,000	45,017	50,846	48,745	11,346

Lower Lake Redding Study Site Boards In WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	31,549	29,232	6,820	16,464
3,500	34,199	31,441	7,876	16,593
3,750	36,516	33,359	8,978	16,572
4,000	38,595	35,040	10,161	16,593
4,250	40,481	36,527	11,303	17,013
4,500	42,270	37,970	12,391	17,229
4,750	43,929	39,296	13,372	17,962
5,000	45,782	40,912	14,330	19,265
5,250	47,592	42,528	15,203	20,224
5,500	49,068	43,821	16,044	20,709
6,000	51,385	45,847	17,595	21,517
6,500	53,012	47,215	18,942	22,034
7,000	54,187	48,174	20,063	22,595
7,500	55,145	48,971	21,032	23,177
8,000	56,104	49,747	21,765	23,769
9,000	56,449	50,060	22,703	24,728
10,000	58,787	52,085	23,963	24,373
11,000	62,386	55,372	26,635	25,547
12,000	64,994	57,193	29,297	25,245
13,000	67,569	58,744	31,064	24,879
14,000	69,788	59,876	32,400	24,254
15,000	71,523	60,522	33,564	23,511
17,000	73,215	59,994	35,320	21,884
19,000	72,579	57,311	36,182	20,429
21,000	69,972	53,077	36,246	19,071
23,000	66,028	47,980	35,600	17,929
25,000	61,373	42,539	34,415	17,046
27,000	56,492	37,281	32,626	16,485
29,000	51,730	32,454	30,288	16,119
31,000	47,161	28,165	27,821	15,828

Bridge Riffle Study Site WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	3,416	4,525	3,200	1,562
3,500	3,453	4,649	3,267	1,684
3,750	3,069	4,312	2,971	1,647
4,000	3,088	4,353	2,807	1,766
4,250	3,307	4,589	2,851	1,766
4,500	3,794	5,081	3,022	1,961
4,750	4,291	5,539	3,295	2,028
5,000	4,839	6,079	3,314	2,155
5,250	5,182	6,378	3,215	2,402
5,500	5,145	6,352	3,313	2,370
6,000	5,882	6,957	3,575	2,578
6,500	6,499	7,514	4,167	2,653
7,000	6,667	7,646	4,495	2,697
7,500	6,791	7,698	4,736	2,725
8,000	7,153	7,973	5,320	2,660
9,000	6,624	7,406	6,067	2,506
10,000	4,909	6,519	10,446	2,116
11,000	3,987	5,691	9,652	1,823
12,000	3,506	5,193	8,512	1,534
13,000	3,109	4,722	7,385	1,214
14,000	2,971	4,392	6,382	1,018
15,000	2,889	4,101	5,475	985
17,000	2,695	3,617	4,797	881
19,000	2,555	3,095	4,167	908
21,000	2,230	2,644	3,436	905
23,000	2,475	2,659	3,255	901
25,000	2,047	2,178	2,197	787
27,000	2,100	2,106	2,400	622
29,000	448	868	1,687	112
31,000	318	608	1,477	125

Posse Grounds Study Site WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	32,594	58,647	30,730	8,318
3,500	35,428	64,035	34,016	8,501
3,750	37,949	68,474	37,011	8,491
4,000	40,050	72,094	39,824	8,447
4,250	41,688	74,766	42,367	8,329
4,500	42,701	76,232	44,791	8,286
4,750	43,315	77,137	47,269	8,254
5,000	43,272	76,867	49,489	8,243
5,250	43,207	76,350	51,859	8,297
5,500	42,560	74,960	53,993	8,534
6,000	40,599	71,060	57,839	8,814
6,500	38,046	66,621	60,878	9,019
7,000	35,525	62,742	62,968	9,245
7,500	33,025	59,455	64,563	9,331
8,000	30,557	56,341	65,435	9,266
9,000	26,506	51,234	65,349	9,266
10,000	22,950	46,019	62,860	8,394
11,000	20,052	41,612	59,197	7,305
12,000	17,595	37,496	54,564	6,053
13,000	14,912	33,854	49,607	4,946
14,000	13,005	31,010	45,211	4,148
15,000	11,367	28,413	41,278	3,459
17,000	8,932	23,855	34,307	2,554
19,000	6,950	19,610	27,724	1,681
21,000	5,937	16,227	22,713	1,153
23,000	4,816	12,876	18,597	813
25,000	3,933	9,676	15,300	638
27,000	2,985	6,691	12,628	584
29,000	1,659	3,556	10,215	398
31,000	1,065	2,047	8,340	255

Above Hawes Hole Study Site WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	75,294	91,651	84,905	22,940
3,500	77,277	93,277	87,168	23,866
3,750	78,839	94,420	89,355	24,340
4,000	79,669	94,894	90,993	24,200
4,250	80,693	95,616	92,879	24,157
4,500	81,371	95,939	94,549	24,243
4,750	81,393	95,659	95,723	24,179
5,000	81,317	95,379	96,575	24,039
5,250	80,682	94,624	96,725	23,985
5,500	80,617	94,312	97,221	24,254
6,000	79,604	93,288	97,189	24,082
6,500	78,354	91,521	96,736	23,726
7,000	75,725	88,817	95,971	22,950
7,500	71,868	85,078	94,926	22,131
8,000	67,192	80,757	93,935	21,463
9,000	58,270	72,848	94,948	20,192
10,000	48,326	63,875	94,646	18,885
11,000	39,331	55,242	89,952	17,495
12,000	32,114	47,693	83,867	15,717
13,000	26,349	41,387	77,192	13,910
14,000	21,311	35,706	70,592	11,968
15,000	17,675	31,101	62,942	9,841
17,000	13,845	26,125	52,391	7,246
19,000	12,061	23,028	44,034	5,367
21,000	11,192	20,900	37,879	4,216
23,000	9,707	18,135	33,473	3,065
25,000	7,841	15,001	29,257	2,247
27,000	6,787	12,846	25,495	1,952
29,000	5,996	11,086	22,330	1,730
31,000	5,171	9,494	19,463	1,474

Powerline Riffle Study Site WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	47,420	60,414	74,206	10,764
3,500	45,427	58,540	75,402	10,727
3,750	43,649	56,826	76,566	10,650
4,000	41,354	54,617	77,191	10,443
4,250	39,177	52,495	77,557	10,287
4,500	36,591	49,995	77,439	10,057
4,750	34,135	47,625	77,094	9,833
5,000	31,624	45,179	76,361	9,592
5,250	29,146	42,711	75,337	9,357
5,500	26,862	40,449	74,282	9,119
6,000	22,530	36,096	71,308	8,534
6,500	18,974	32,454	67,720	7,827
7,000	16,033	29,307	63,787	6,950
7,500	13,587	26,625	59,649	6,033
8,000	11,610	24,308	55,286	5,103
9,000	8,791	20,698	47,097	3,404
10,000	7,010	18,048	39,748	2,110
11,000	6,004	16,184	33,833	1,414
12,000	5,330	14,621	29,200	1,025
13,000	4,869	13,275	25,332	782
14,000	4,395	11,949	22,164	533
15,000	3,958	10,623	19,276	339
17,000	3,551	8,458	15,031	238
19,000	3,149	6,616	12,068	217
21,000	2,745	5,007	9,726	227
23,000	1,733	3,159	7,904	196
25,000	967	1,751	6,614	143
27,000	637	1,088	5,408	71
29,000	. 376	713	4,392	25
31,000	266	463	3,537	0.11

Price Riffle Study Site WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	38,078	63,162	48,110	5,183
3,500	40,298	66,276	52,894	6,260
3,750	40,729	66,384	54,402	7,106
4,000	40,686	65,974	55,727	7,730
4,250	40,082	65,241	57,925	8,254
4,500	39,468	64,703	60,285	8,915
4,750	38,552	63,787	62,731	9,603
5,000	38,024	63,280	65,522	10,253
5,250	37,852	63,140	66,858	10,904
5,500	37,119	62,612	68,183	11,335
6,000	35,050	60,414	69,390	11,971
6,500	33,456	58,701	69,702	12,714
7,000	32,669	57,257	70,500	13,188
7,500	32,712	56,288	69,702	13,081
8,000	32,421	55,210	68,851	12,887
9,000	34,318	54,779	64,972	12,790
10,000	35,869	55,016	65,478	13,565
11,000	33,844	52,085	67,030	13,328
12,000	31,452	49,133	67,558	13,996
13,000	27,206	43,789	66,330	14,341
14,000	25,084	40,686	65,522	14,374
15,000	24,416	38,391	64,390	14,320
17,000	20,806	33,111	58,109	11,626
19,000	15,009	25,547	49,402	8,552
21,000	11,626	20,677	42,323	6,930
23,000	8,788	15,419	33,402	4,831
25,000	6,493	11,928	28,155	3,722
27,000	5,050	9,203	23,597	3,130
29,000	4,727	7,844	20,278	2,938
31,000	4,364	6,767	17,972	2,898

APPENDIX G REVISED SEGMENT SPAWNING HABITAT MODELING RESULTS

Segment 6 Boards Out WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	279,956	268,782	372,493	67,290
3,500	272,448	261,253	388,925	65,436
3,750	263,774	252,885	402,996	63,843
4,000	253,776	243,641	415,056	62,510
4,250	244,038	234,676	424,360	61,329
4,500	233,851	225,351	431,541	60,386
4,750	223,342	215,899	437,056	59,758
5,000	209,927	204,667	442,398	59,303
5,250	200,688	196,217	443,784	59,004
5,500	190,763	187,812	444,708	58,850
6,000	170,499	170,393	439,809	58,699
6,500	150,726	155,371	433,948	58,155
7,000	132,377	140,981	422,490	57,329
7,500	115,717	128,082	408,786	56,242
8,000	101,484	116,970	392,377	54,809
9,000	79,126	98,976	356,827	50,758
10,000	62,728	85,662	321,193	45,616
11,000	51,351	75,706	286,447	39,310
12,000	42,696	67,373	254,558	32,430
13,000	36,917	61,455	224,684	25,210
14,000	32,772	56,614	198,570	18,296
15,000	29,176	52,149	174,819	12,521
17,000	25,704	45,751	136,442	5,818
19,000	26,113	42,715	109,349	5,501
21,000	26,971	40,391	90,598	6,581
23,000	28,733	39,528	79,200	8,223
25,000	41,981	50,939	77,130	14,618
27,000	58,462	67,199	89,683	16,277
29,000	65,025	73,542	111,038	15,394
31,000	58,425	66,546	129,813	14,313

Segment 6 Boards In WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	229,136	191,251	104,203	117,637
3,500	247,661	204,155	117,625	114,496
3,750	264,056	215,391	131,085	111,059
4,000	278,825	225,233	144,731	107,439
4,250	290,741	233,011	157,942	104,371
4,500	300,718	239,302	171,156	101,194
4,750	308,861	243,951	183,920	98,865
5,000	315,465	247,691	195,976	97,430
5,250	320,542	250,242	207,214	95,832
5,500	324,159	251,621	218,183	93,805
6,000	327,630	250,639	236,196	90,417
6,500	325,301	246,486	253,990	87,742
7,000	319,436	239,902	268,517	86,016
7,500	310,948	231,989	280,713	84,760
8,000	300,743	224,054	292,079	83,899
9,000	277,576	204,986	303,219	83,593
10,000	256,327	189,577	307,267	82,855
11,000	238,470	177,870	307,201	84,251
12,000	220,851	166,374	303,648	83,876
13,000	205,842	156,696	295,249	83,428
14,000	192,505	148,170	283,910	82,611
15,000	182,153	140,914	271,951	82,097
17,000	163,256	126,497	244,438	79,760
19,000	146,967	112,883	215,785	77,114
21,000	132,965	99,213	186,998	73,916
23,000	121,006	86,254	159,577	70,348
25,000	110,053	73,820	132,926	66,074
27,000	99,515	62,537	108,407	61,106
29,000	90,279	52,018	85,780	56,262
31,000	82,157	42,909	69,615	52,360

Segment 5 WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	517,564	493,885	569,220	152,613
3,500	540,135	516,656	596,120	158,337
3,750	557,335	533,387	619,524	160,323
4,000	571,053	546,578	640,059	160,020
4,250	584,449	558,157	661,485	159,272
4,500	594,577	565,434	681,914	160,379
4,750	599,845	568,889	700,715	160,244
5,000	601,840	568,857	715,521	160,132
5,250	600,180	565,753	727,117	161,281
5,500	596,697	560,241	740,184	163,485
6,000	586,295	546,463	759,708	164,954
6,500	571,480	528,443	774,931	164,601
7,000	548,314	507,864	782,849	162,248
7,500	519,331	485,617	786,638	158,970
8,000	487,794	462,776	788,865	155,259
9,000	425,010	419,447	796,884	148,633
10,000	354,260	371,357	804,490	136,687
11,000	294,671	327,119	760,657	123,797
12,000	247,450	288,319	703,857	108,364
13,000	206,321	255,082	642,741	93,326
14,000	173,385	226,835	585,266	79,673
15,000	148,479	202,932	525,439	66,425
17,000	118,445	170,974	438,261	49,667
19,000	100,282	145,888	363,681	36,995
21,000	90,019	126,869	306,694	29,174
23,000	79,041	107,407	265,007	22,222
25,000	64,268	85,667	223,952	17,075
27,000	55,205	69,041	194,105	14,685
29,000	37,679	49,477	163,971	10,416
31,000	30,476	38,755	140,251	8,621

Segment 4 WUA (ft²)

Flow (cfs)	Fall-run	Late-fall-run	Winter-run	Steelhead
3,250	276,159	594,401	183,474	51,509
3,500	276,892	600,365	192,444	54,868
3,750	272,541	592,640	196,452	57,352
4,000	264,989	580,043	199,377	58,699
4,250	256,007	566,310	203,223	59,887
4,500	245,671	551,697	206,586	61,280
4,750	234,779	535,892	209,738	62,778
5,000	224,963	521,688	212,825	64,099
5,250	216,404	509,143	213,293	65,443
5,500	206,659	495,723	213,698	66,066
6,000	185,983	464,213	211,047	66,231
6,500	169,349	438,456	206,133	66,347
7,000	157,307	416,373	201,431	65,046
7,500	149,546	398,812	194,027	61,738
8,000	142,220	382,482	186,206	58,108
9,000	139,242	363,044	168,104	52,307
10,000	138,499	351,438	157,839	50,630
11,000	128,709	328,374	151,295	47,617
12,000	118,806	306,657	145,137	48,518
13,000	103,602	274,478	137,493	48,847
14,000	95,217	253,174	131,529	48,150
15,000	91,648	235,757	125,499	47,349
17,000	78,673	199,947	109,710	38,321
19,000	58,650	154,704	92,205	28,324
21,000	46,418	123,540	78,074	23,117
23,000	33,983	89,360	61,959	16,237
25,000	24,096	65,796	52,154	12,484
27,000	18,369	49,500	43,508	10,339
29,000	16,483	41,159	37,005	9,570
31,000	14,955	34,776	32,264	9,361